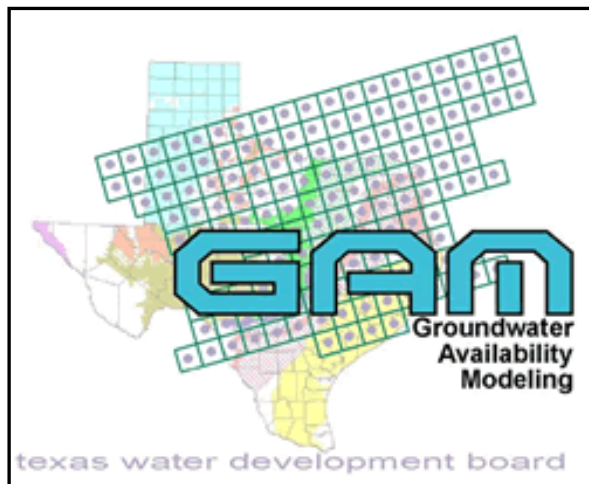

Groundwater Availability Modeling (GAM) for the Dockum Aquifer

Stakeholders Advisory Forum 3
Model Calibration Results
June 4, 2008



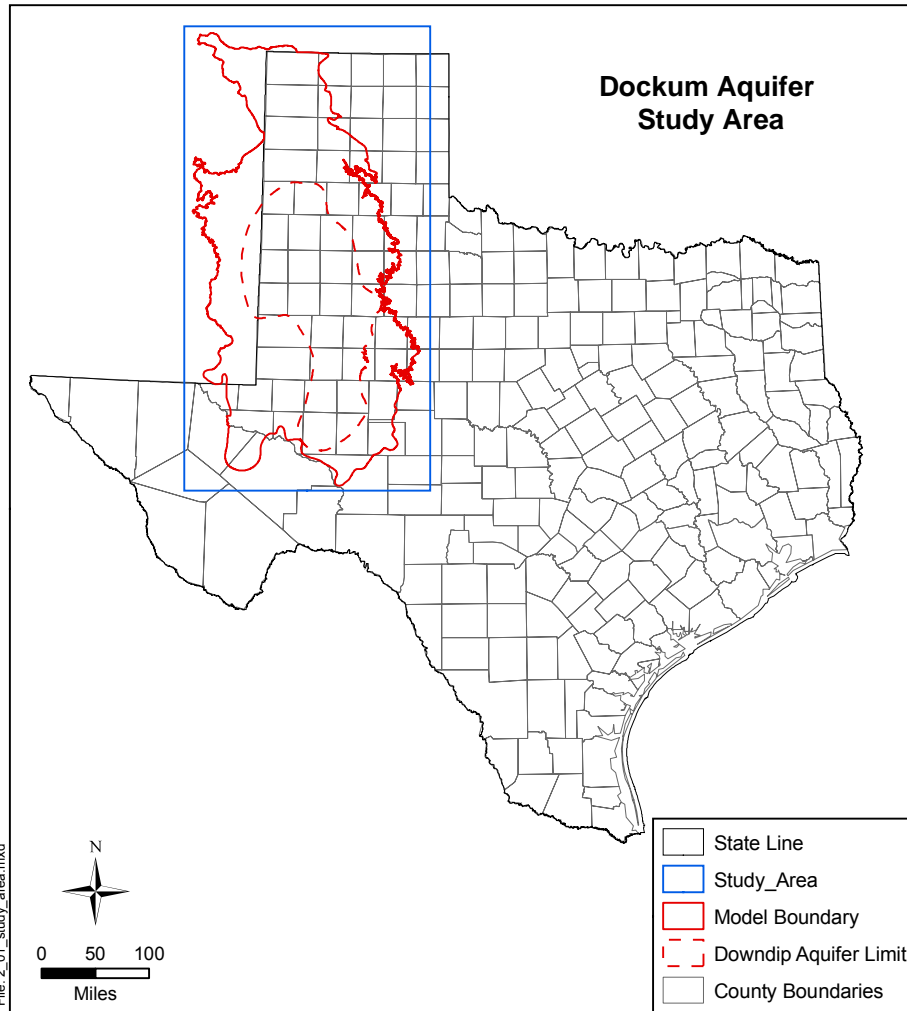
Outline

- **Aquifer Review**
- **Model Design**
- **Conceptual Model**
- **Model Implementation**
- **Model Calibration**
- **Model Results**
- **Model Conclusions**
- **Model Limitations**
- **GAM Schedule**

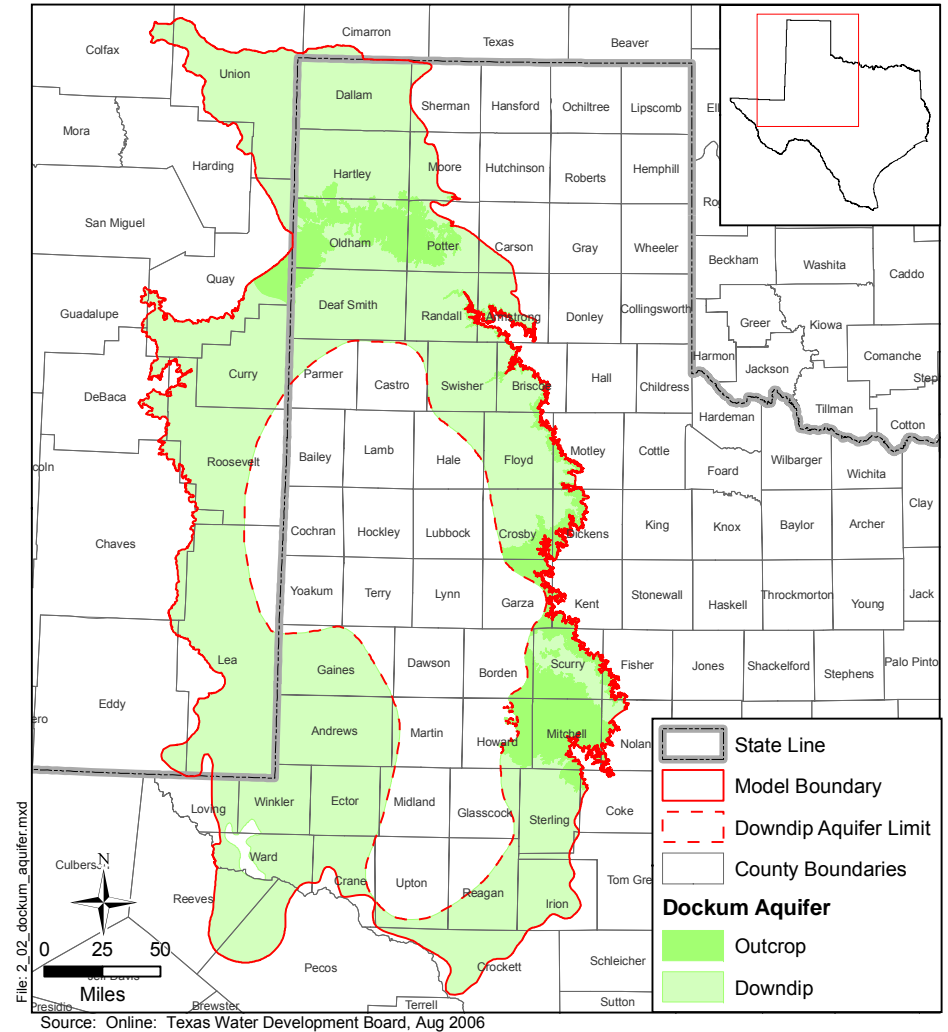
Aquifer Review

Aquifer Location

Study Area

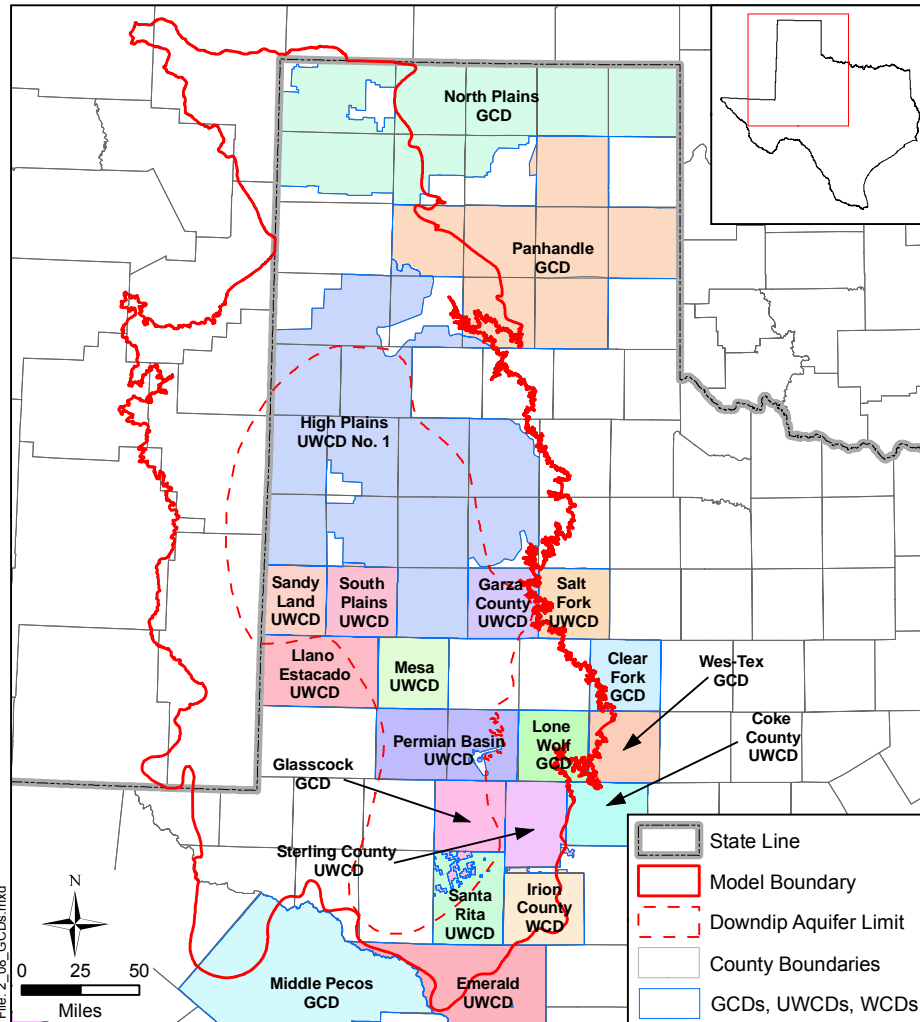


Model Domain

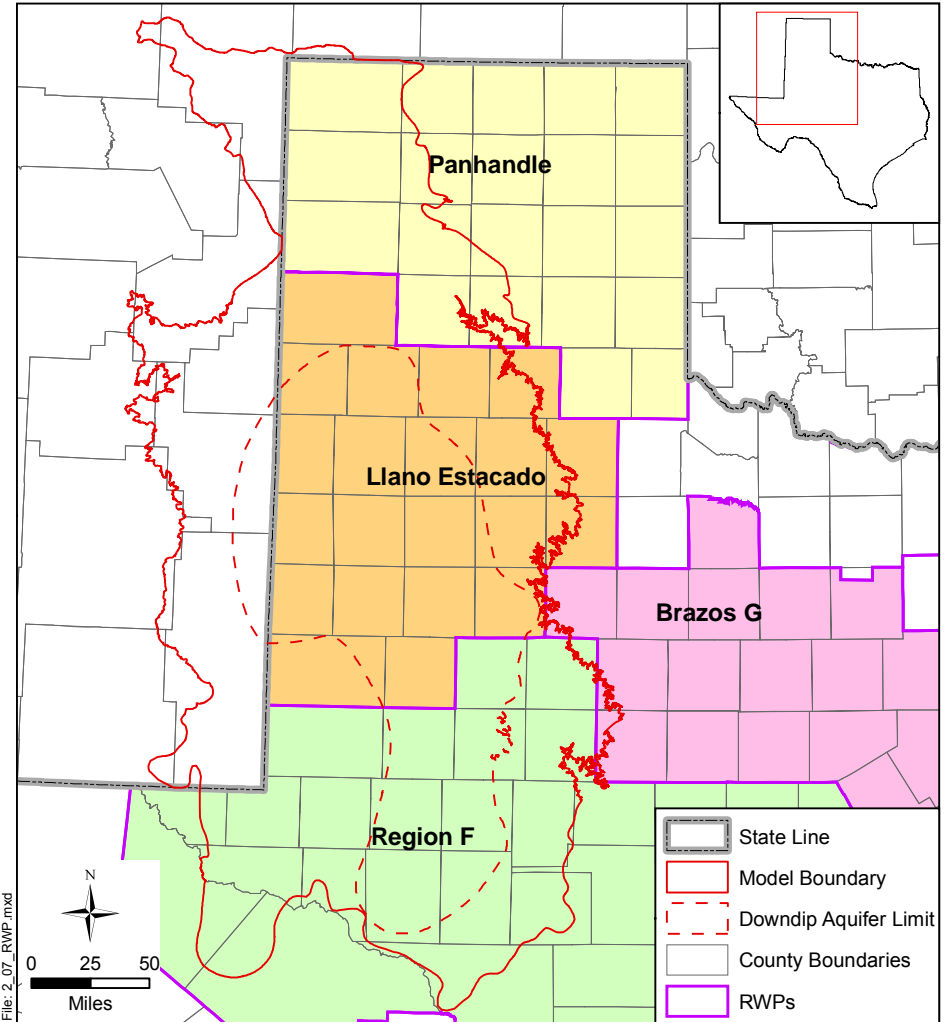


Aquifer Location

Groundwater Conservation Districts

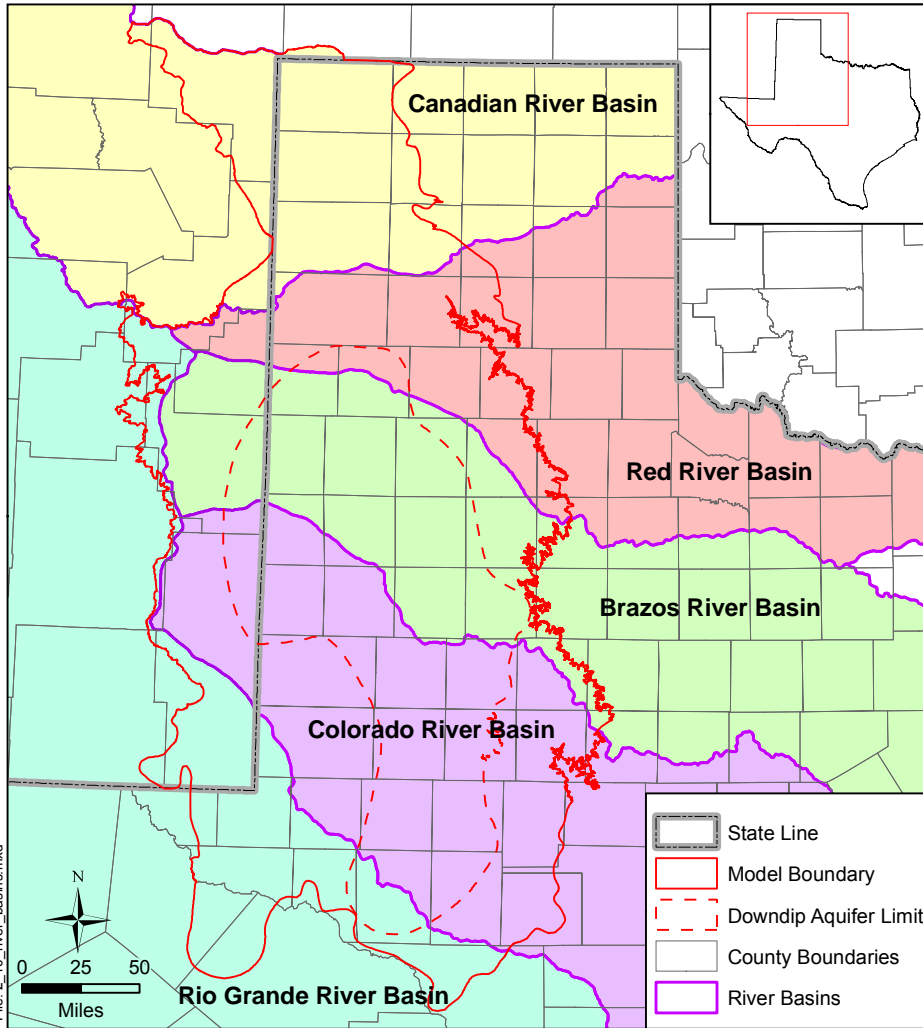


Regional Water Planning Groups



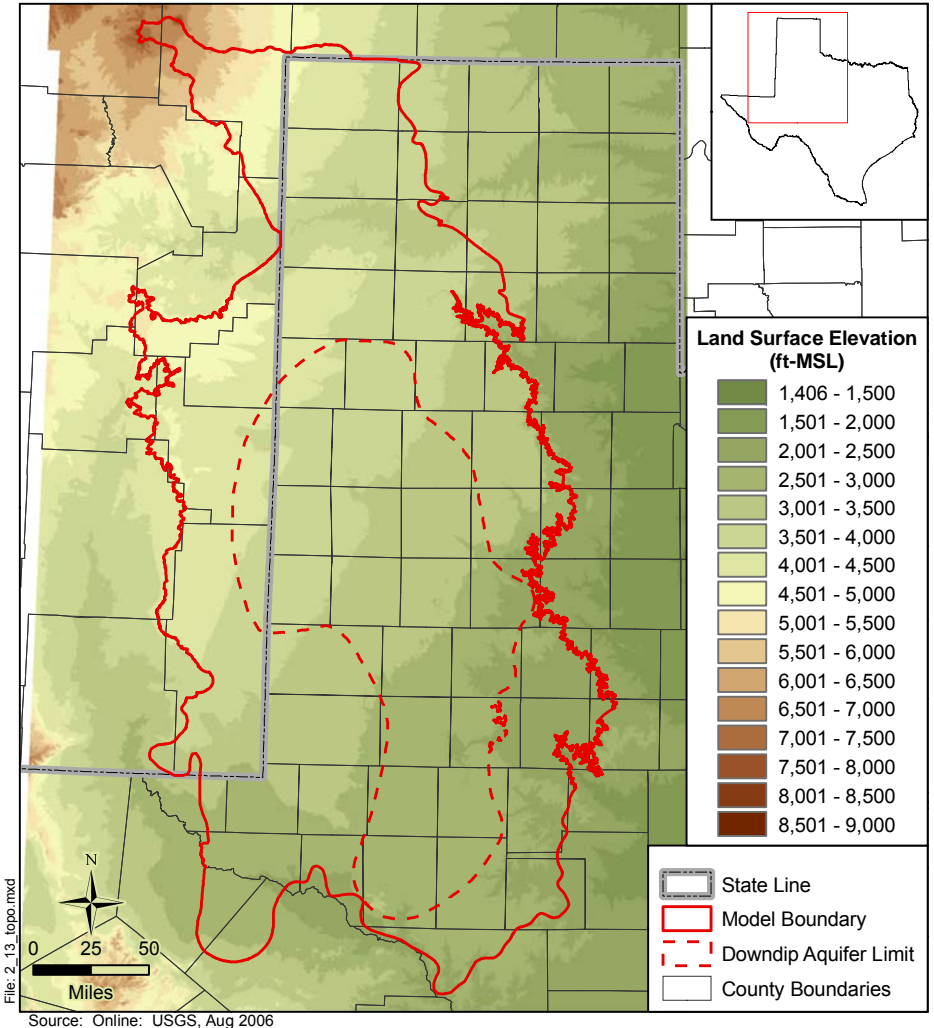
River Basins and Topography

Major River Basins



Source: Online: Texas Water Development Board, June 2006; New Mexico Resource GIS Program, June 2006

Topography



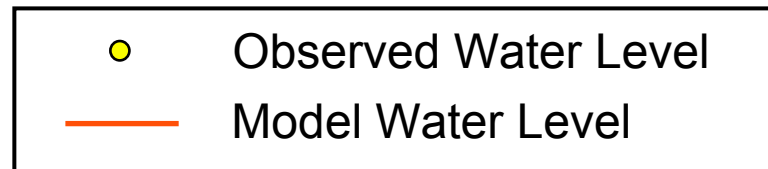
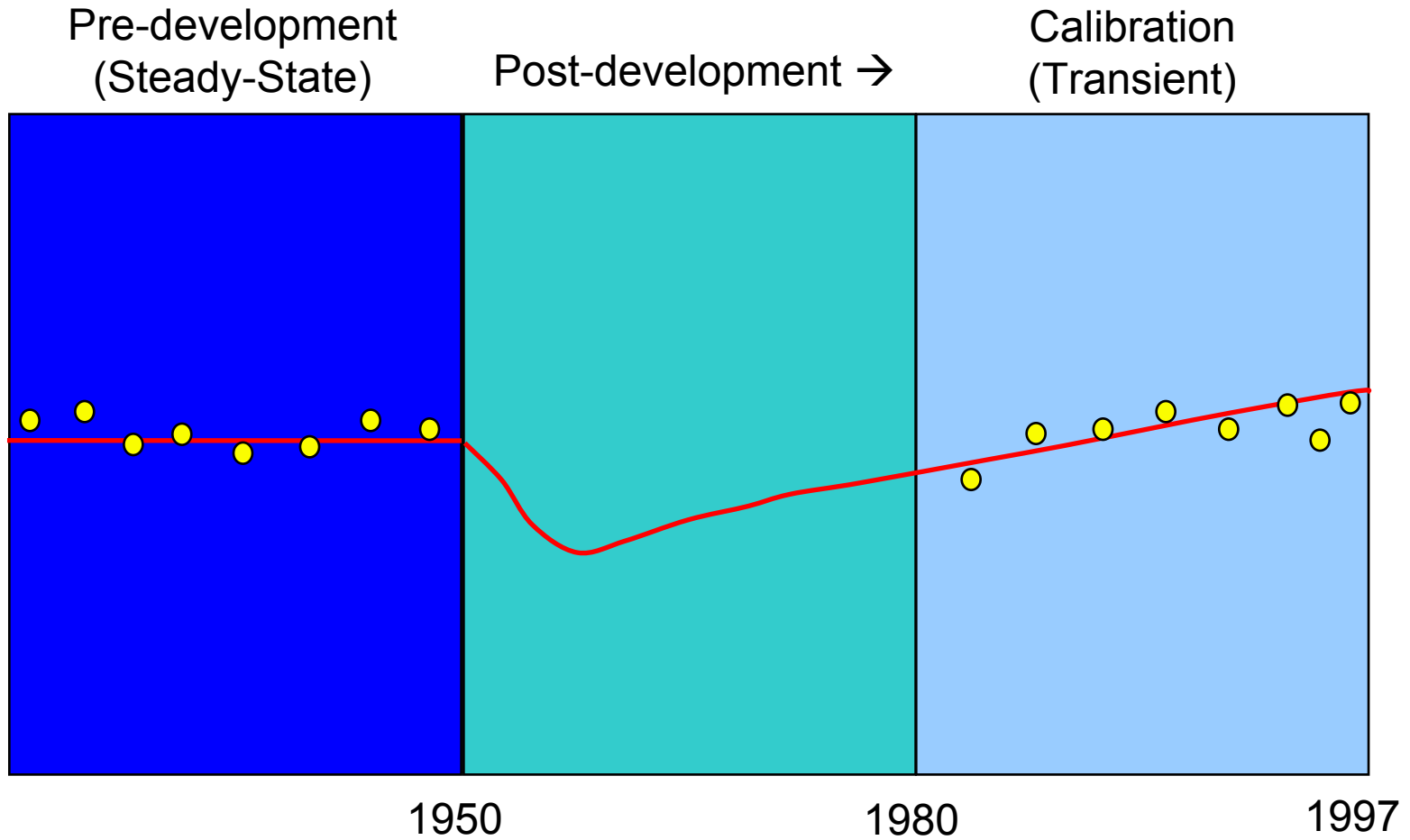
Source: Online: USGS, Aug 2006

Model Design

Modeling Protocol

- Compile and Analyze Field and Literature Data
- Develop a Conceptual Model
- Model Design
- Calibrate the Model
- Use the Model for Predictive Purposes

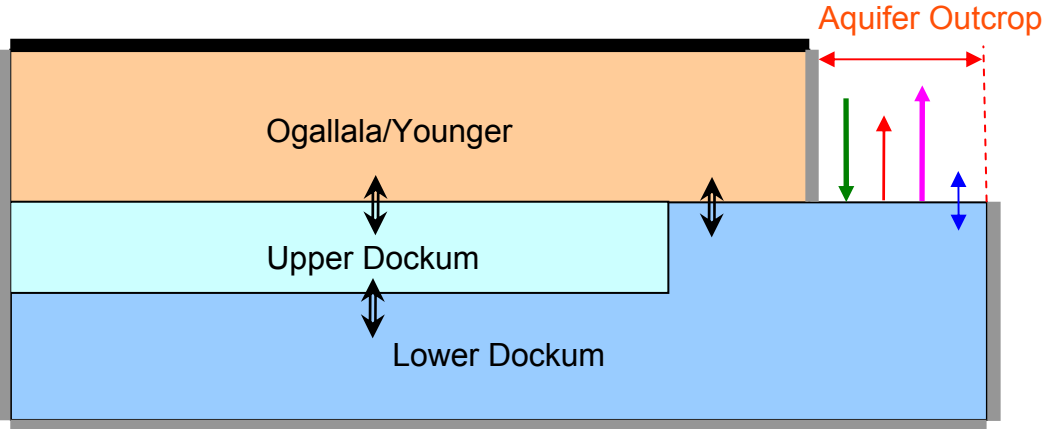
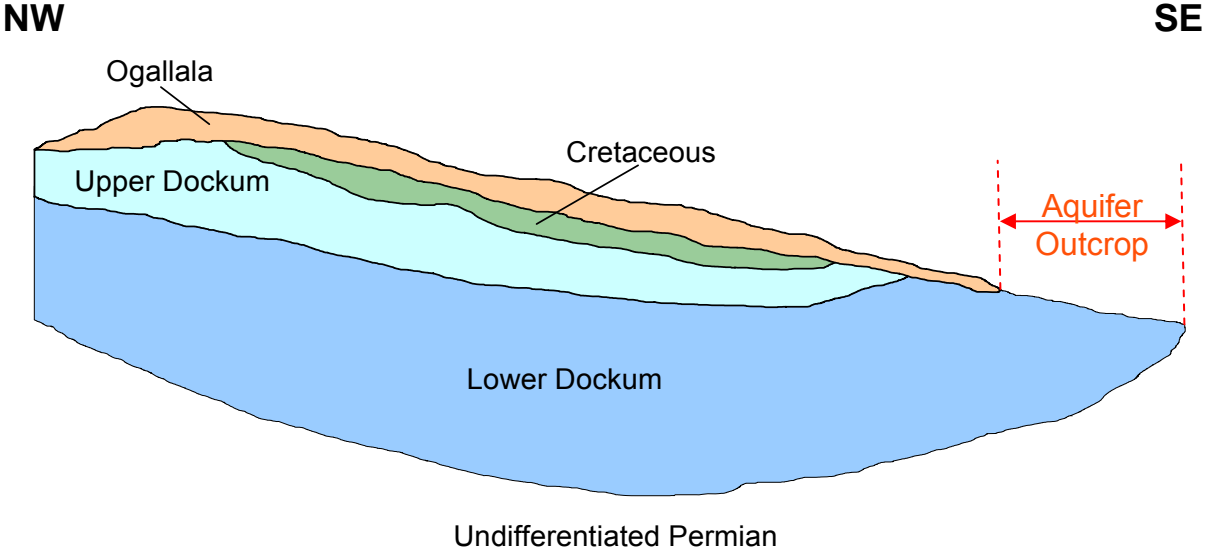
Schematic of Modeling Periods












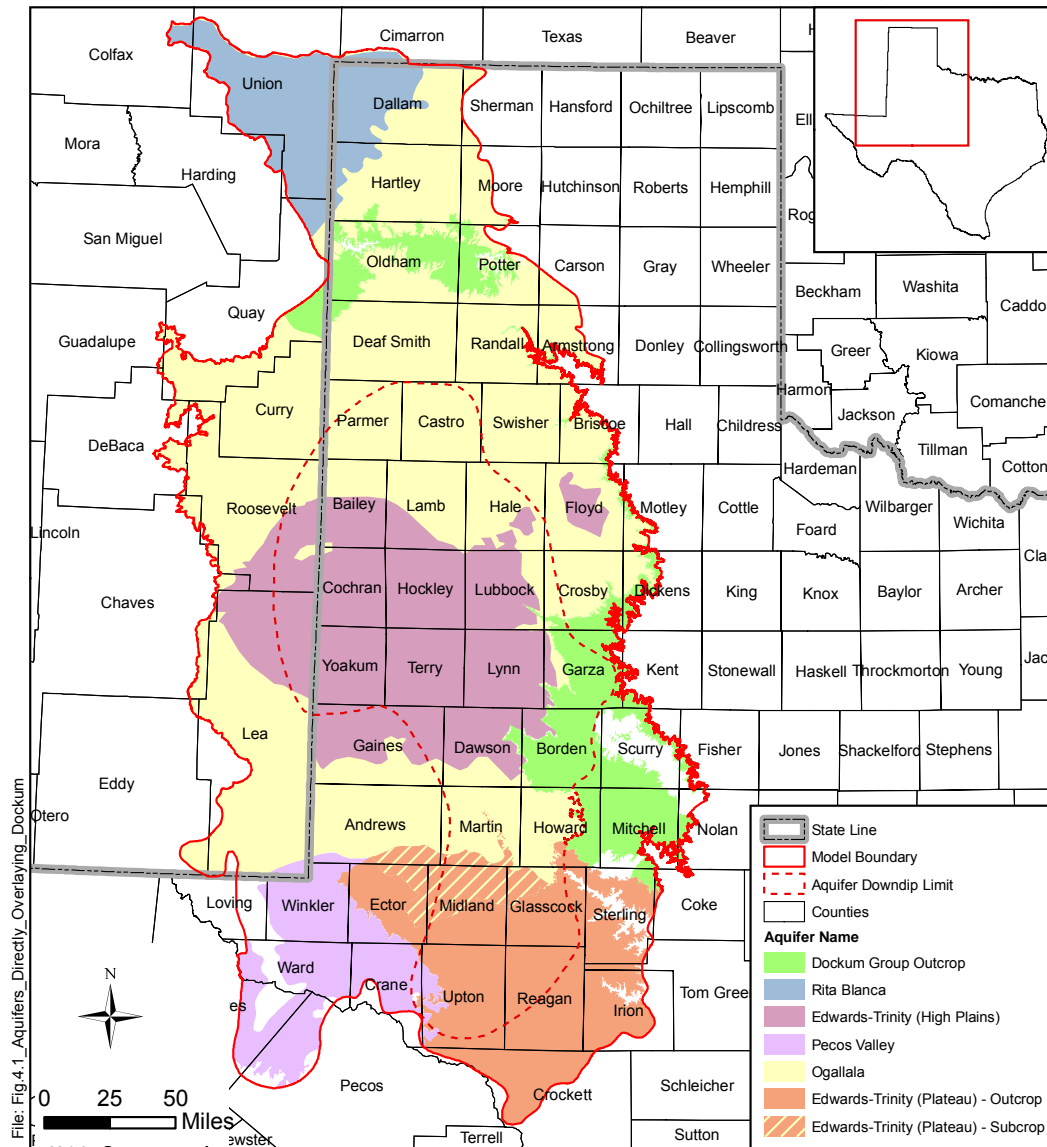
Conceptual Model

Conceptualization of Groundwater Flow



-  General Head Boundary
-  Recharge (Precipitation)
-  Discharge (ET, Springs)
-  Discharge via Pumping
-  Surface Water-Aquifer Interaction
-  Cross-Formational Flow
-  No-Flow Boundary

Aquifer Directly Overlying the Dockum

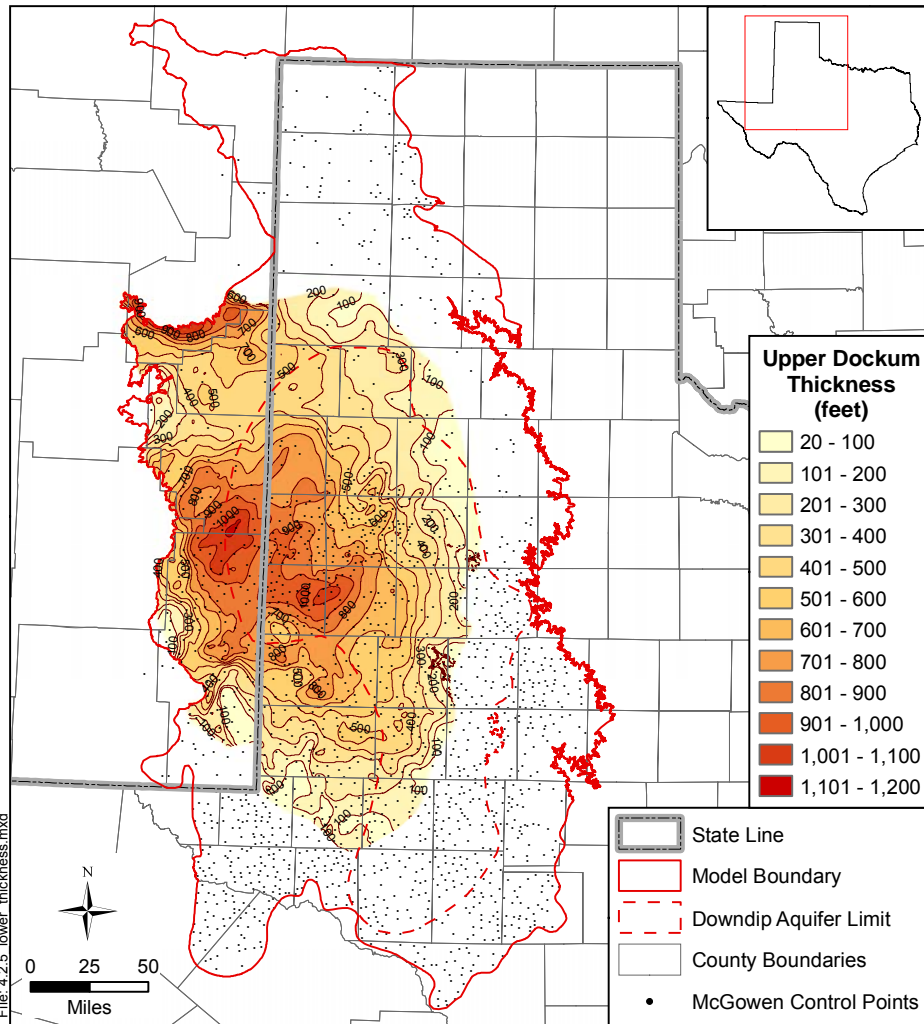


File: Fig.4.1_Aquifers_Directly_Overlying_Dockum

Source: Adapted from Texas Water Development Board, Dec 2006

Dockum Thickness

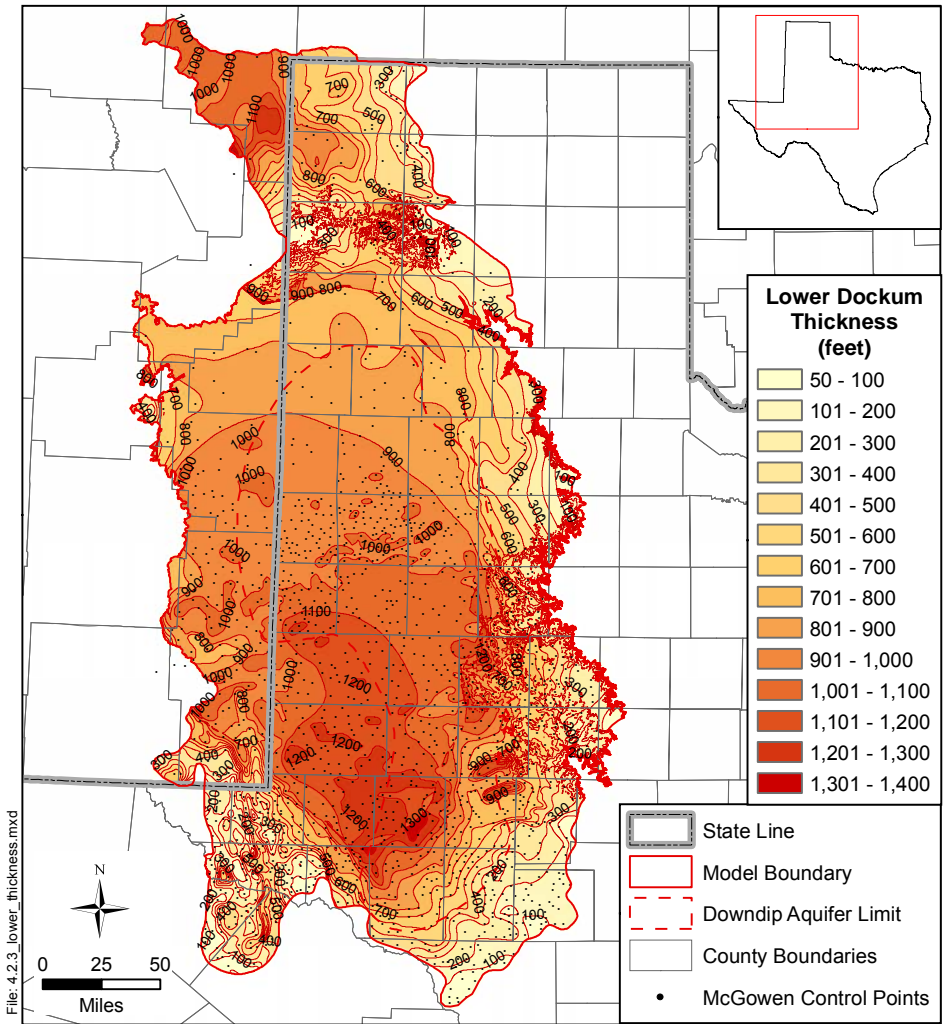
Upper Dockum



File: 4.2.5_lower_thickness.mxd

Source: McGowen et al. (1977)

Lower Dockum

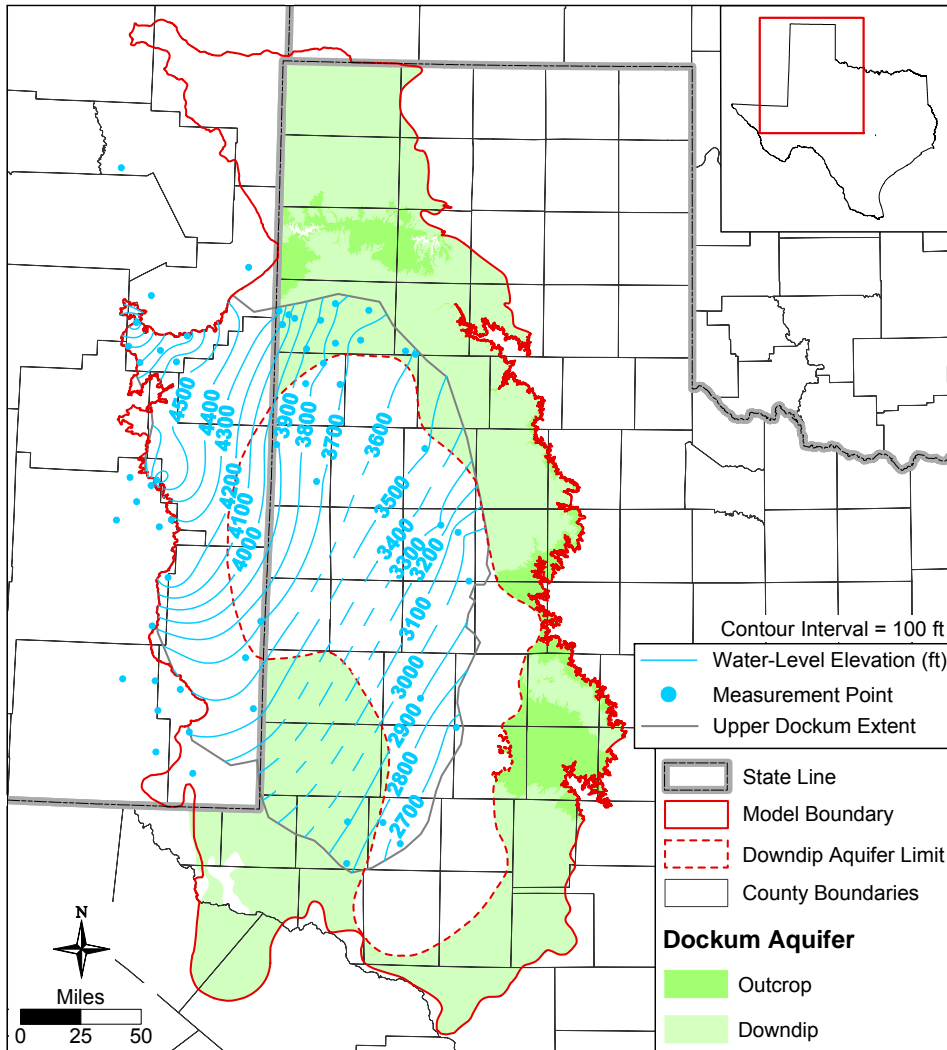


File: 4.2.3_lower_thickness.mxd

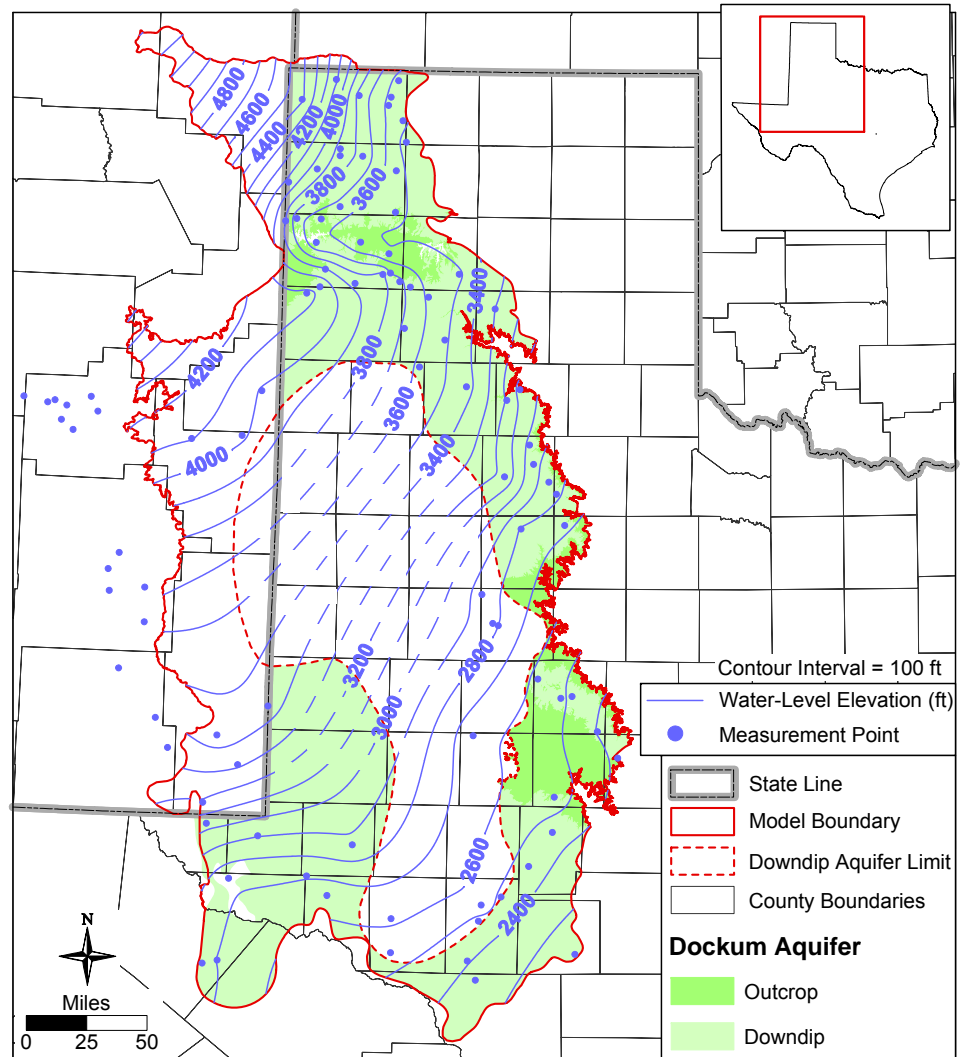
Source: McGowen et al. (1977)

Predevelopment Water-Level Elevations

Upper Dockum

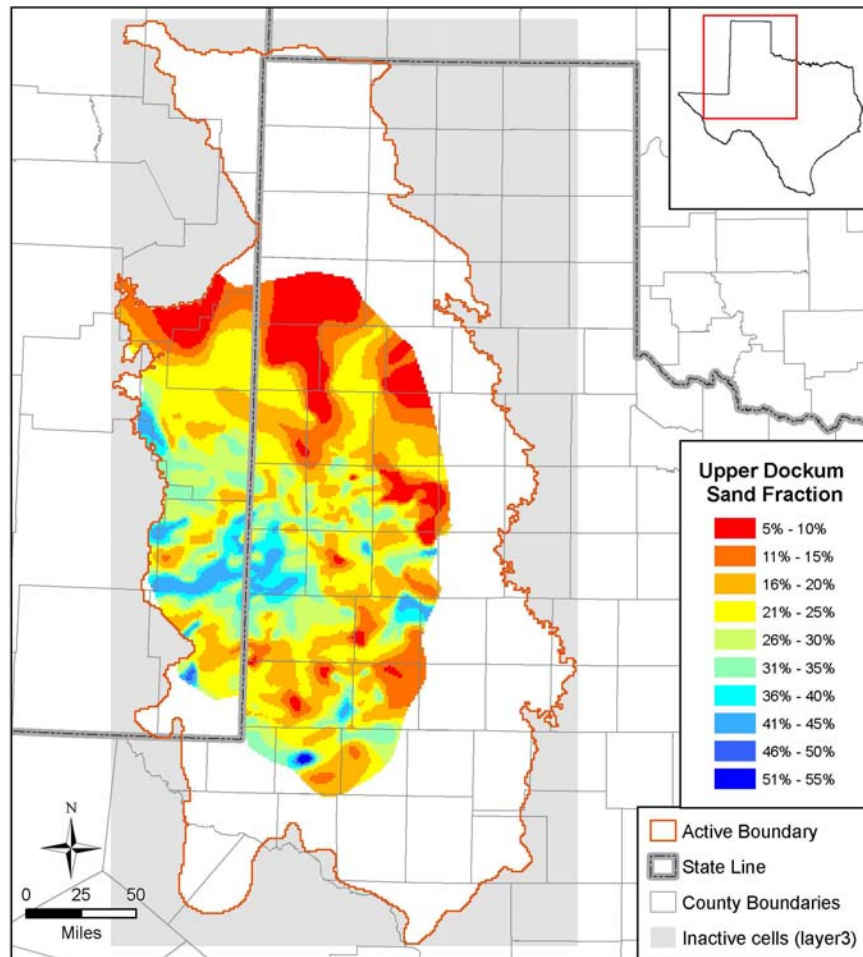


Lower Dockum

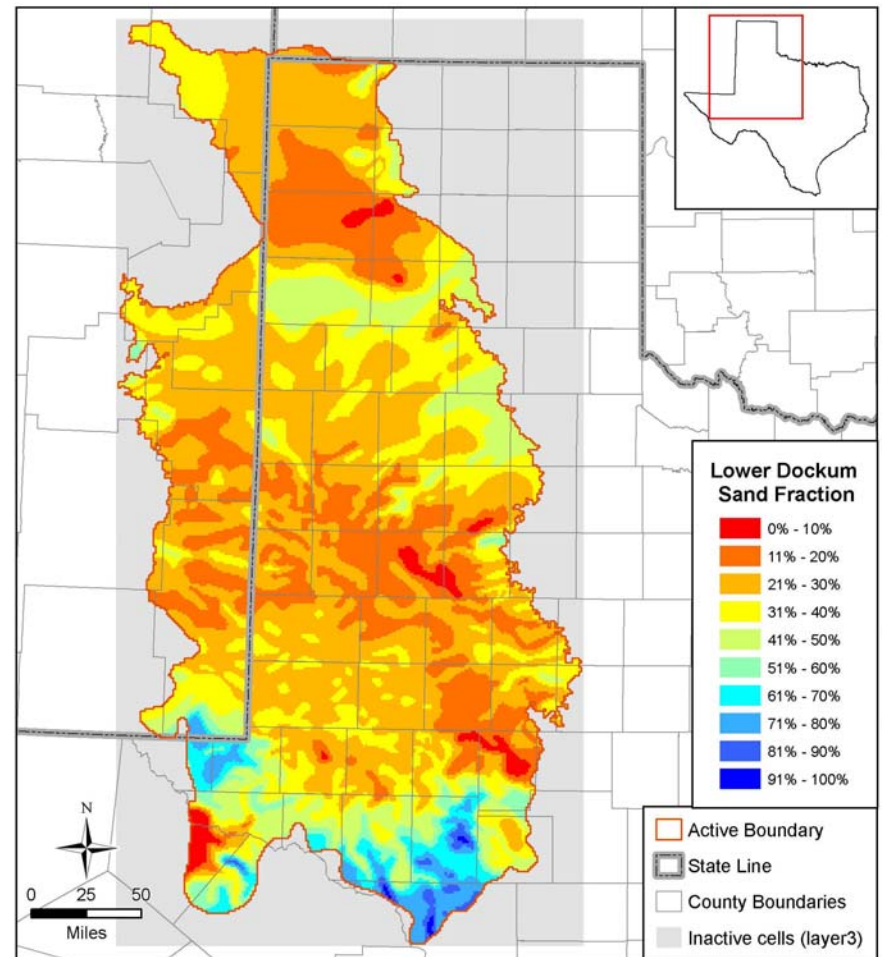


Sand Fraction

Upper Dockum

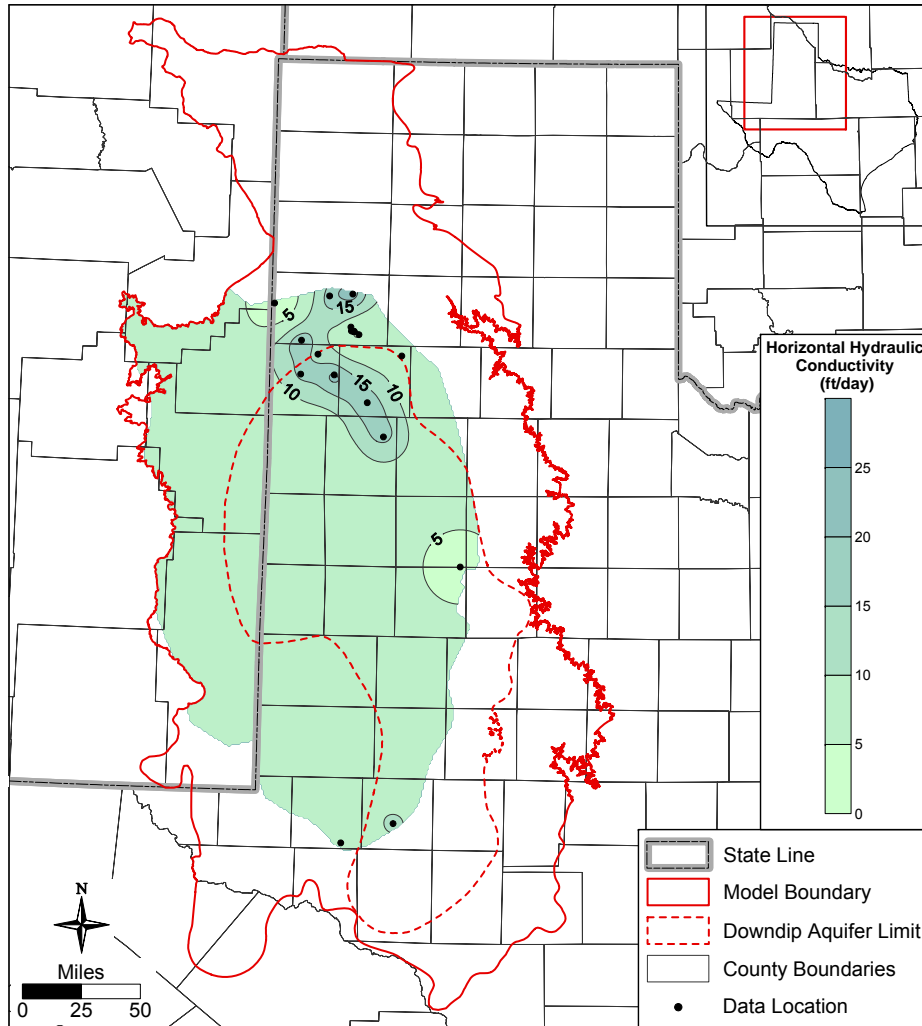


Lower Dockum

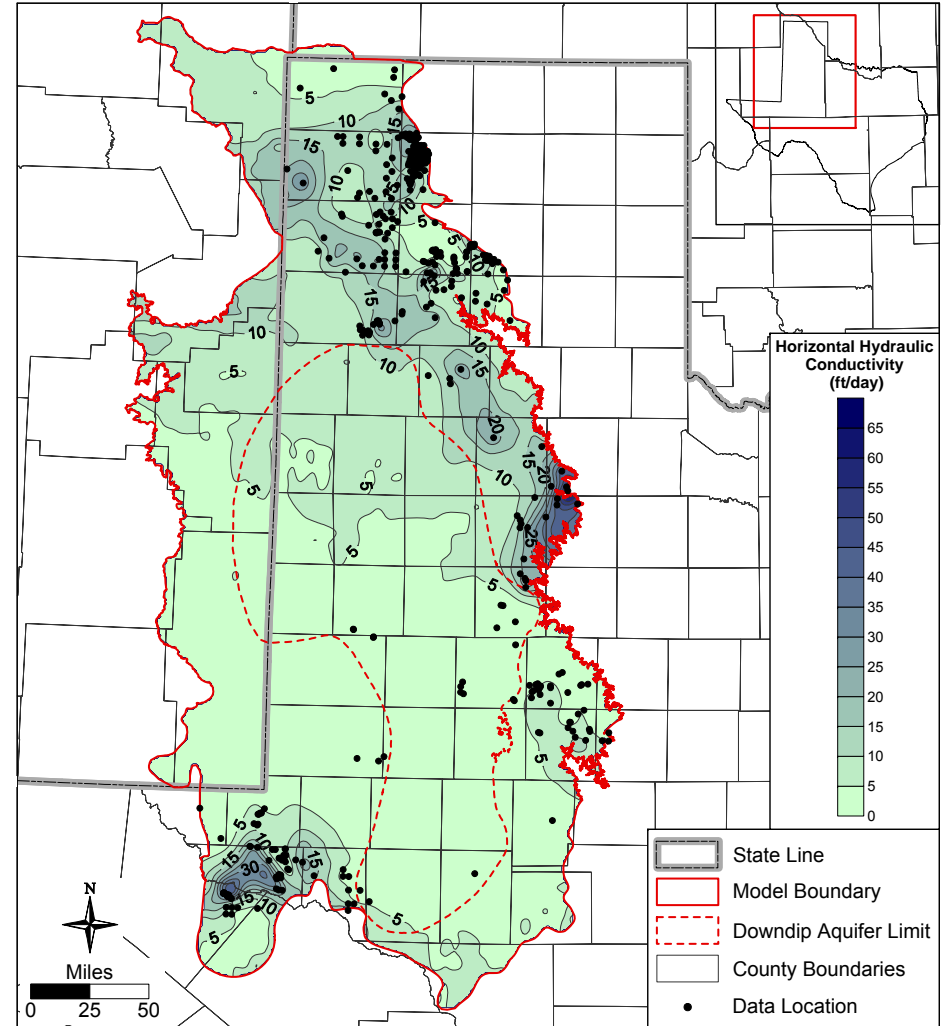


Horizontal Sand Hydraulic Conductivity

Upper Dockum



Lower Dockum



Conceptual Model of Groundwater Flow

■ Outcrop Areas

- Recharge by precipitation and stream loss
- Discharges to springs and streams and by ET

■ Subcrop with TDS < 5,000 mg/L

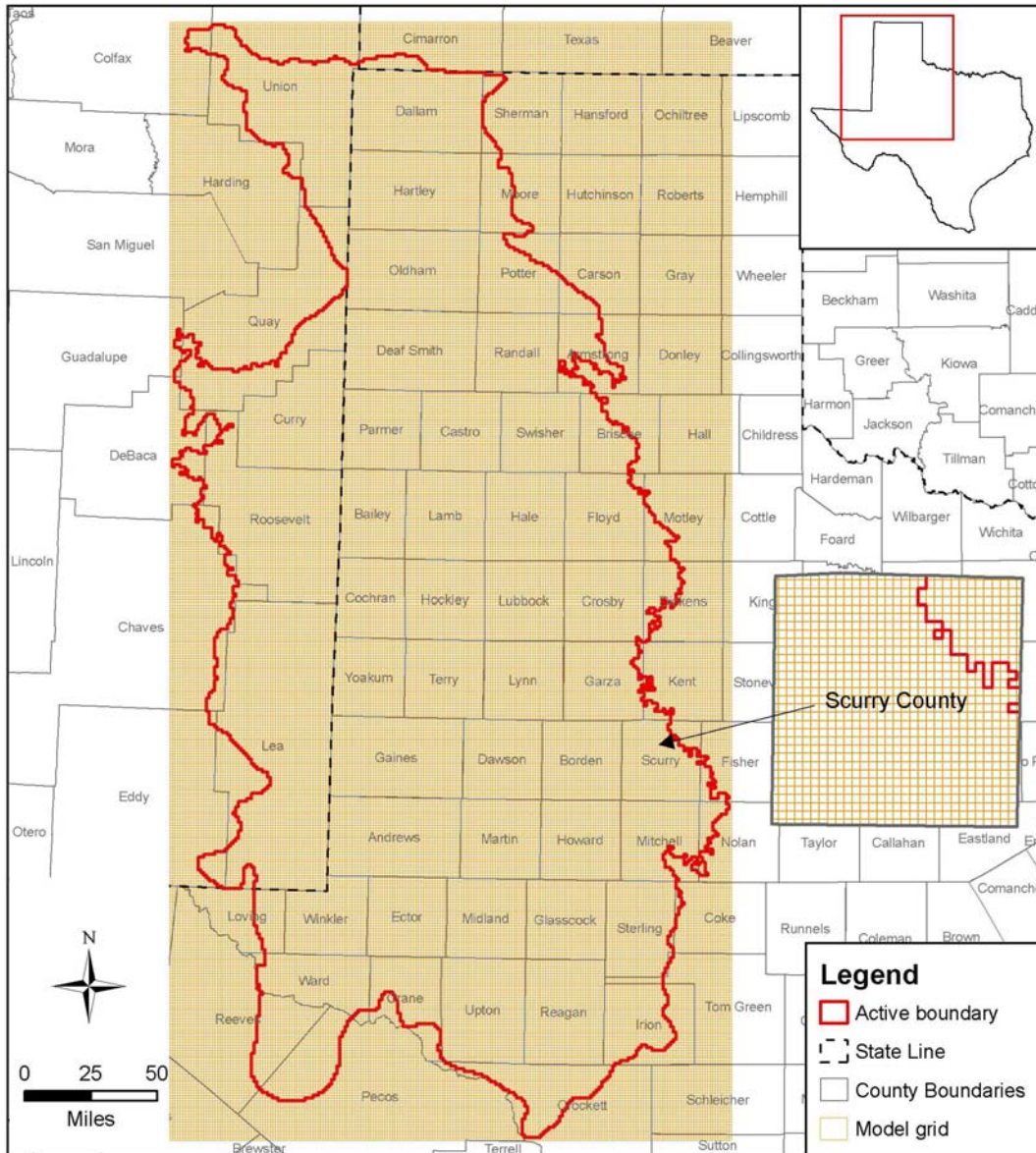
- Portion of Dockum Group defined as an aquifer
- Only lower Dockum present
- Fresh water enters via cross-formational flow from overlying Ogallala and Pecos Valley aquifers
- Flow is towards the Canadian River in the northern portion of the model area
- Flow is towards the southeast along the eastern side of the model area and discharge to springs or overlying formations
- Flow is likely towards the trough in southwestern portion of model area

Conceptual Model of Groundwater Flow

- Subcrop with TDS > 5,000 mg/L
 - Correspond to the portion of the Dockum Group not defined as an aquifer
 - Upper and lower Dockum present
 - Little movement of groundwater into or out of the deeper parts of the depositional basin
 - No or insignificant displacement of connate water by meteoric water
 - Connate water from recharge in eastern New Mexico prior to Pleistocene time when Pecos River valley was eroded
 - Movement of water out of the deeper parts of the basin is restricted by the high fluid density of the groundwater
 - Very little cross-formational flow from overlying aquifers due to the high mudstone content in the upper Dockum

Model Implementation

Model Grid



1 square mile grid blocks
212 columns
422 rows
3 layers
150,548 active cells

Stress Periods

| period | time | type | length |
|---------------|------------------------|-------------|---------------|
| 1 | pre-development | ss | |
| 2 | 1950-1959 | tr | 10 yrs |
| 3 | 1960-1969 | tr | 10 yrs |
| 4 | 1970-1974 | tr | 5 yrs |
| 5 | 1975 | tr | 1 yr |
| . | . | . | . |
| . | . | . | . |
| . | . | . | . |
| 27 | 1997 | tr | 1 yr |

Hydraulic Properties - Dockum

- K_H based on sand hydraulic conductivity and sand fraction:

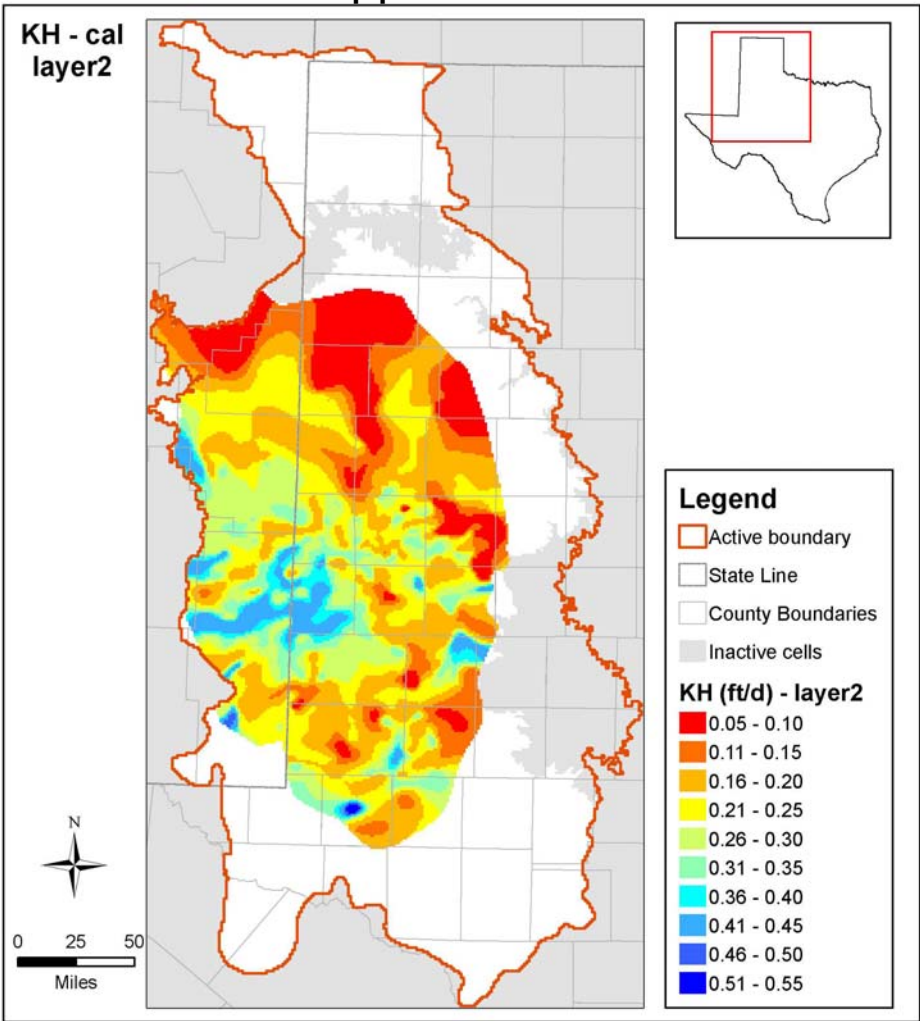
$$K_H = SF \cdot K_{sand}$$

- K_v calculated as harmonic mean of sand and clay conductivities:

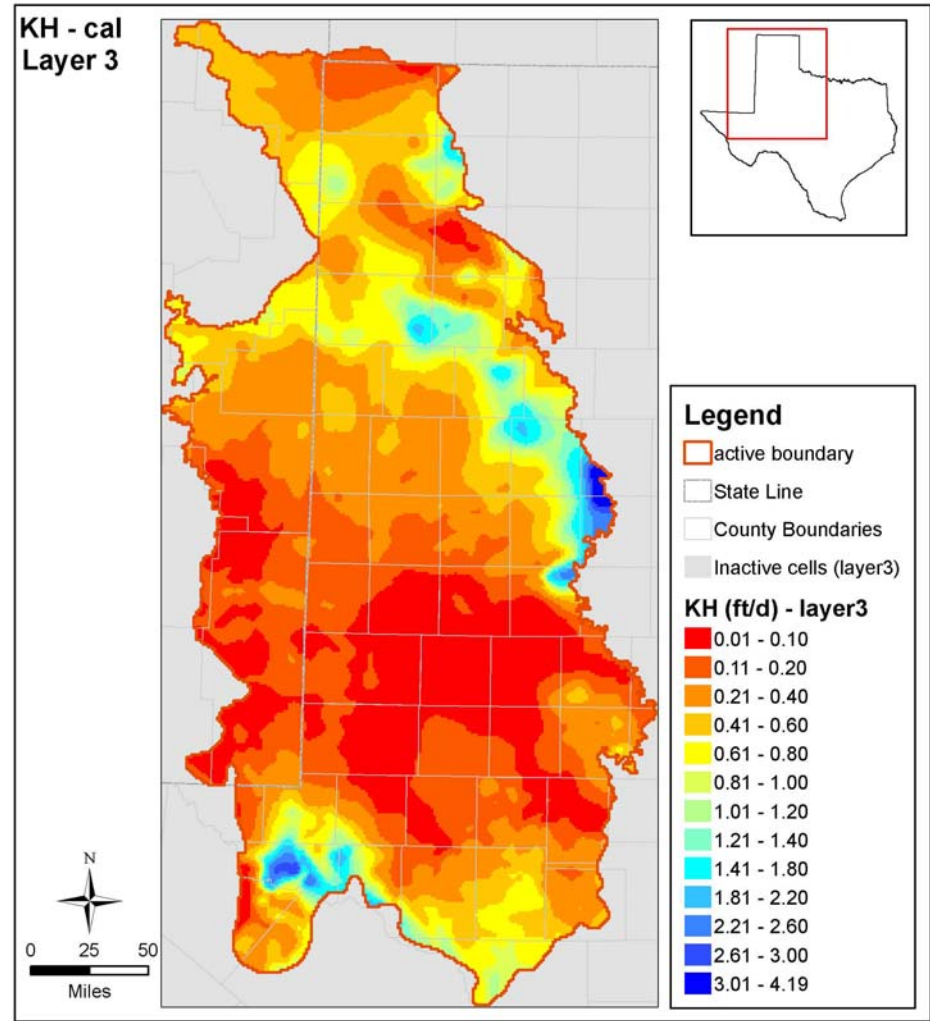
$$K_v = \frac{1}{\frac{SF}{K_{sand}} + \frac{1-SF}{K_{clay}}}$$

Horizontal Hydraulic Conductivity – K_H

Upper Dockum

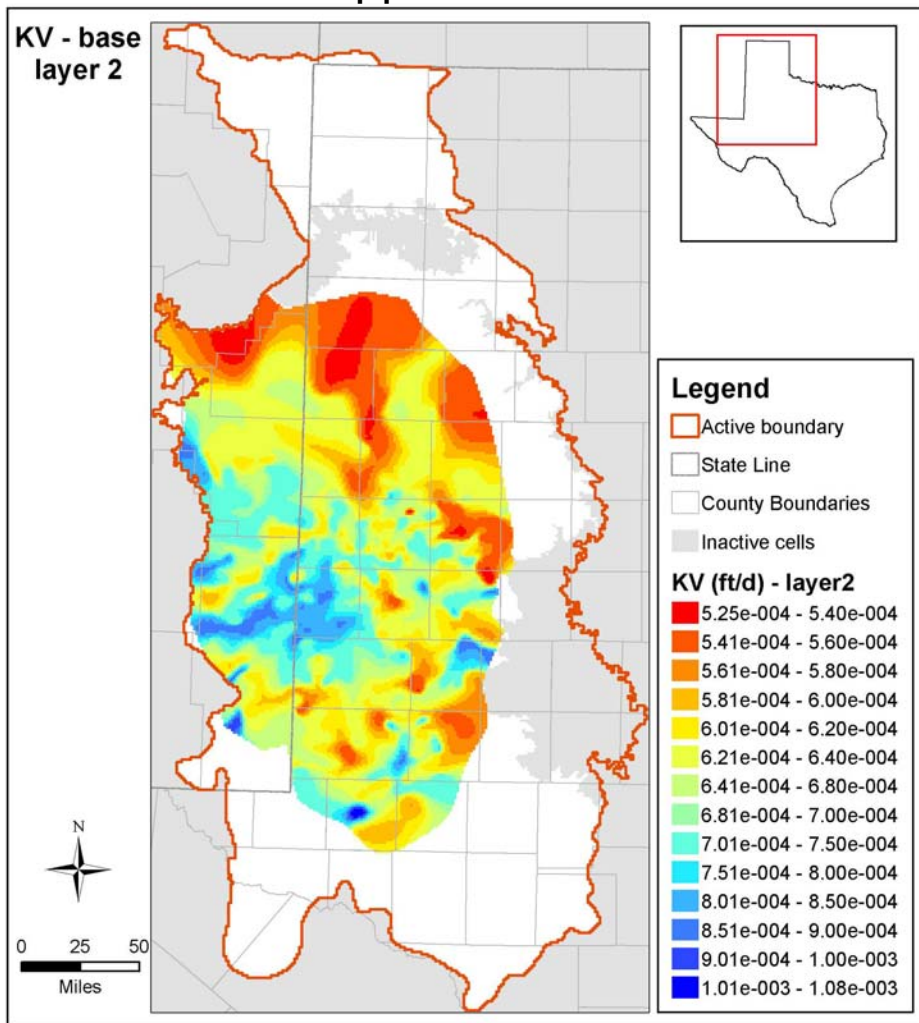


Lower Dockum

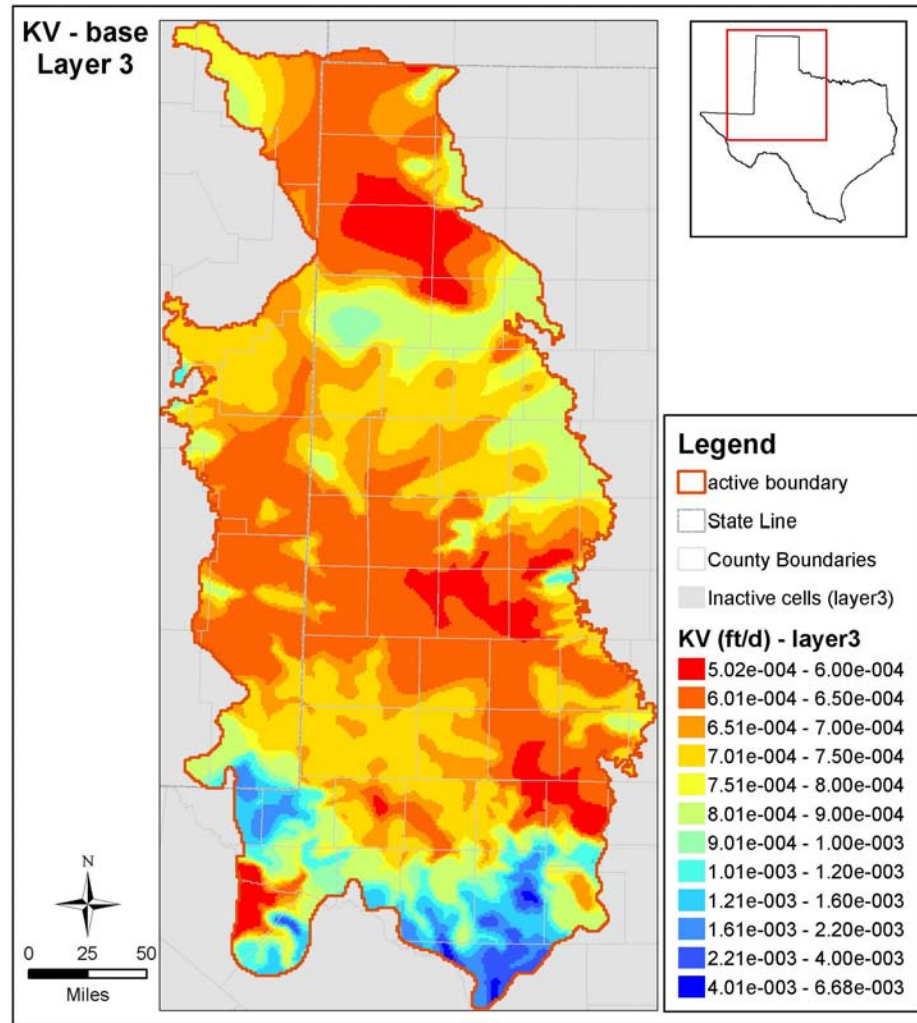


Vertical Hydraulic Conductivity – K_v

Upper Dockum



Lower Dockum



Storage Parameters

- Specific Storage coefficient for upper and lower dockum aquifers are calculated using equation:

$$S_s = SF * S_{s-sand} + (1 - SF) * S_{s-clay}$$

where:

SF – sand fraction

S_{s-sand} – sand specific storage : $3E-6 ft^1$

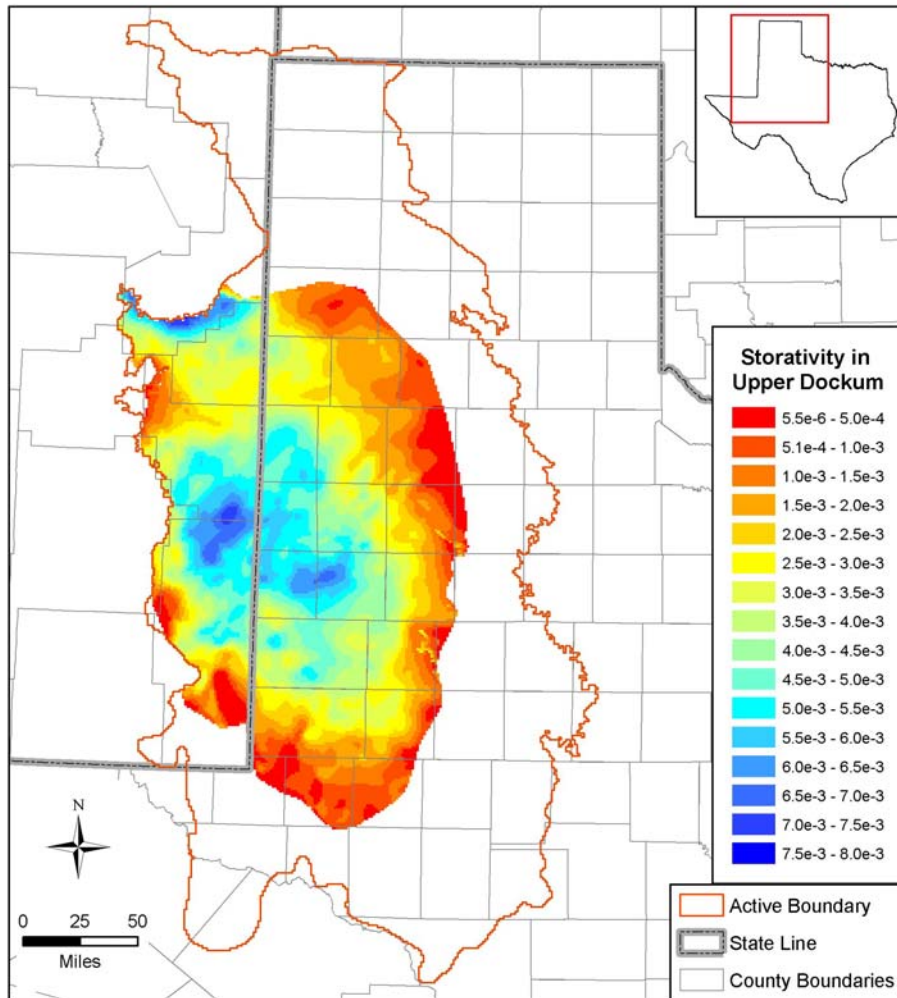
S_{s-clay} – clay specific storage : $7.5E-6 ft^1$

S_{s-min} – water specific storage : $1.3E-6 ft^1$

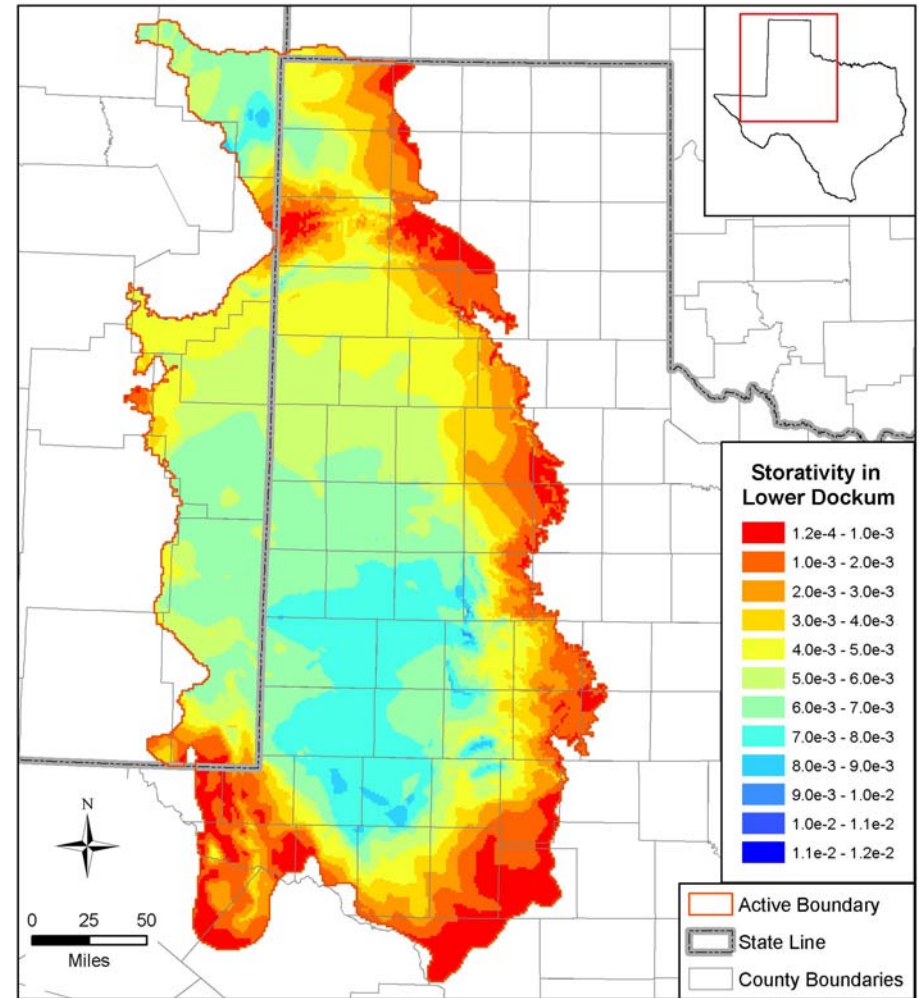
- Storativity product of specific storage and aquifer thickness
- Specific yield set to literature value in absence of data
0.15

Storativity

Upper Dockum



Lower Dockum



Recharge Rate Estimates

| County/Area | Land use | Aquifer | Recharge (in/yr) | Technique | Reference |
|---|-------------------------|---------|---|--|-----------------------|
| <i>Dockum Aquifer – Colorado River outcrop area</i> | | | | | |
| All of the Colorado River outcrop area - Predevelopment | Grassland and shrubland | Dockum | 0.08 | SZ chloride mass balance | This report |
| Sandy areas (Nolan and eastern Mitchell counties) - Predevelopment | Grassland and shrubland | Dockum | 0.22 | SZ chloride mass balance | This report |
| All of the Colorado River outcrop area | | Dockum | 0.08 to 0.2 | UZ numerical modeling | Scanlon et al. (2003) |
| Western Scurry and Mitchell counties | Grassland and shrubland | Dockum | <0.01 | SZ chloride mass balance | This report |
| Scurry County - Predevelopment | | Dockum | 0.02 to 0.04 | Water budget on playas | This report |
| All of the Colorado River Outcrop area - Postdevelopment | | Dockum | 2.2 | regional water level rise | This report |
| Sandy areas (Nolan and eastern Mitchell counties) - Postdevelopment | Cropland | Dockum | Geom. Average = 1.7 Median = 1.6 Range = 0 to 4.3 | linear water level rises in individual wells | This report |

| County/Area | Land use | Aquifer | Recharge (inch/yr) | Technique | Reference |
|--|-------------------------|---------|--------------------|--------------------------|-----------------------|
| <i>Dockum Aquifer – Canadian River outcrop area</i> | | | | | |
| All of the Canadian River outcrop area- Predevelopment and Postdevelopment | Grassland and shrubland | Dockum | 0.17 | SZ chloride mass balance | This report |
| All of the Canadian River outcrop area | | Dockum | <0.08 | UZ numerical modeling | Scanlon et al. (2003) |
| <i>Dockum Aquifer – high TDS outcrop area</i> | | | | | |
| Howard, Borden and Garza counties - Predevelopment | Grassland and shrubland | Dockum | <0.01 | SZ chloride mass balance | This report |

Predevelopment Recharge

- Recharge was estimated on limited points (80) using:

$$R = P \frac{Cl_P}{Cl_{GW}}$$

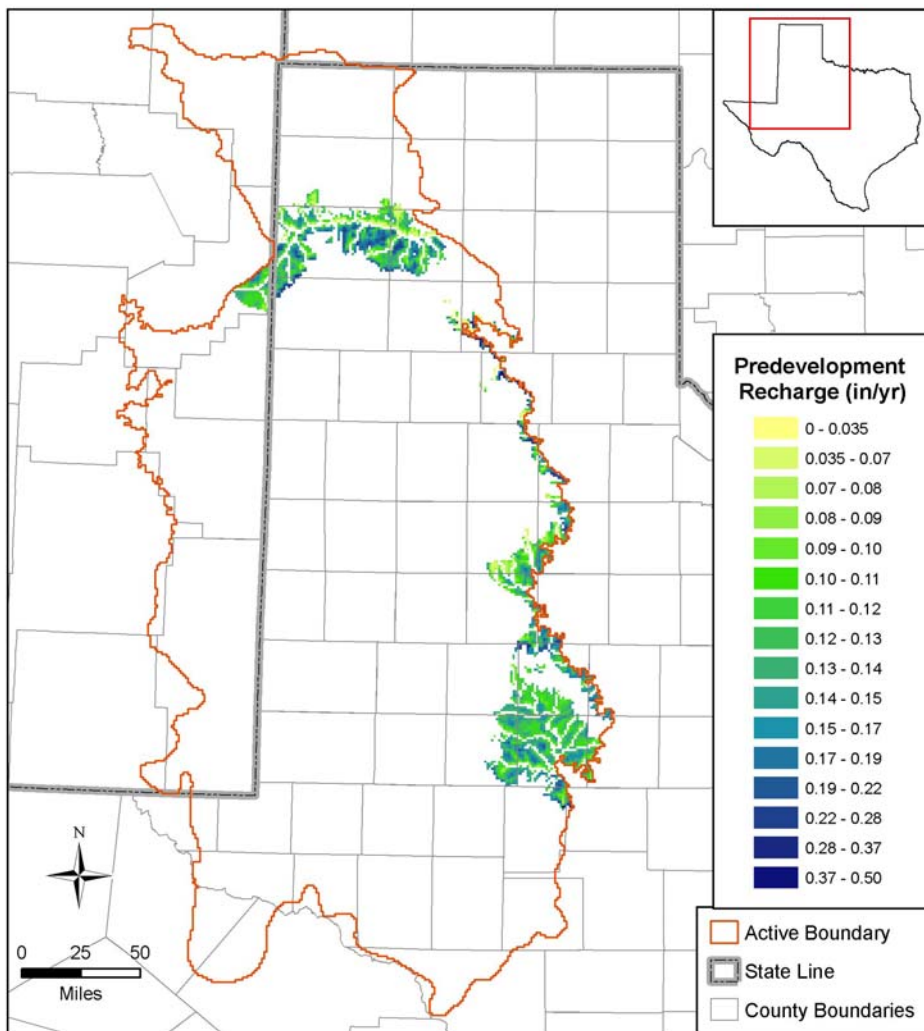
- The correlation of estimated recharge to physical parameters was tested, however, no obvious correlation was found
- A significance analysis (t-test) was conducted to determine whether average recharge rates should be divided into regions; no significant difference existed
- Recharge rates were weighted as a power function of the local topography and normalized to conserve total recharge
- Power coefficient adjusted until maximum recharge rate was reasonable (~0.5 in/yr)

Modern Recharge

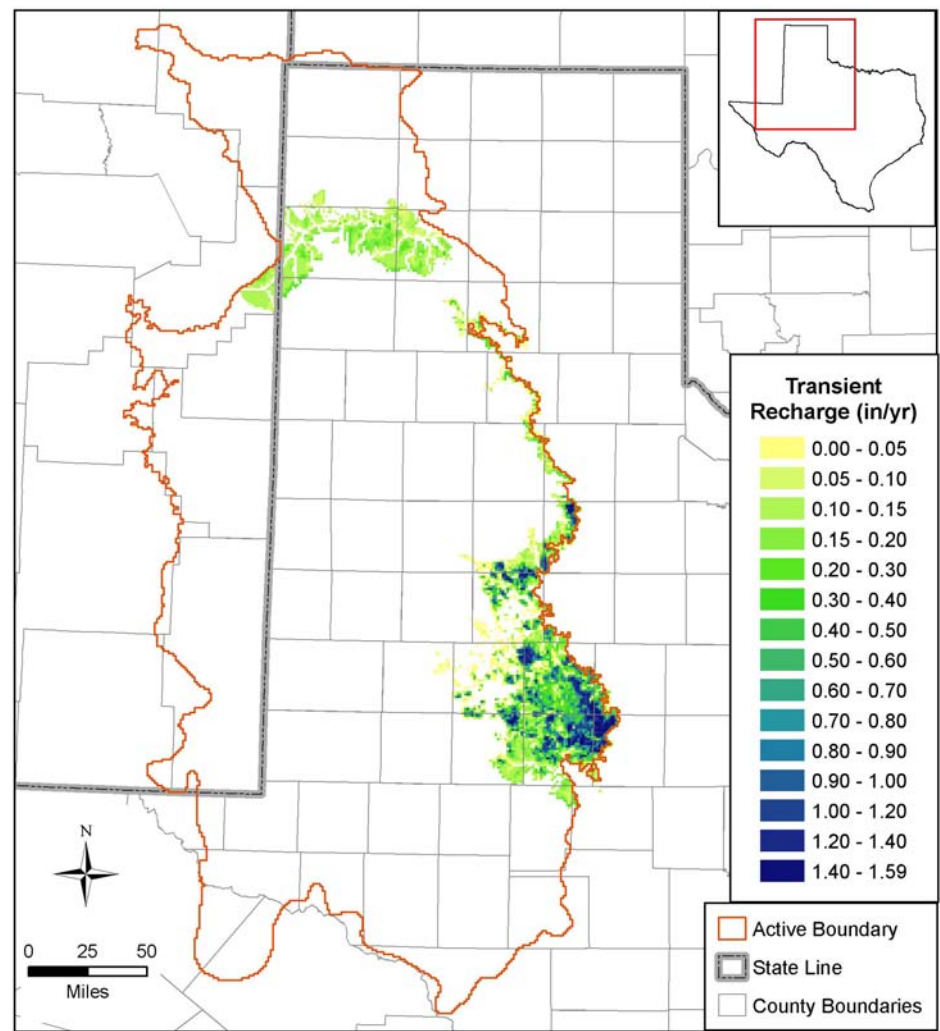
- **Analysis of regional water-level rise was conducted**
 - Colorado River outcrop rise indicates 2.2 in/yr effective recharge
 - Canadian River outcrop indicates no appreciable rise
 - includes **recovery**, **stream loss**, **land-use impacts**, **return flow**
- **For non-pumped, interstream wells with linear water table rise:**
 - Recharge = $\Delta h/Sy\Delta t$ where median = 1.6 in/yr
 - Already have 0.15 in/yr historical recharge
- **Added 1.45 in/yr to cropland areas**
 - Only added within Colorado River outcrop
 - Added to cells by percent cropland within cell

Recharge Distribution

Predevelopment



Modern

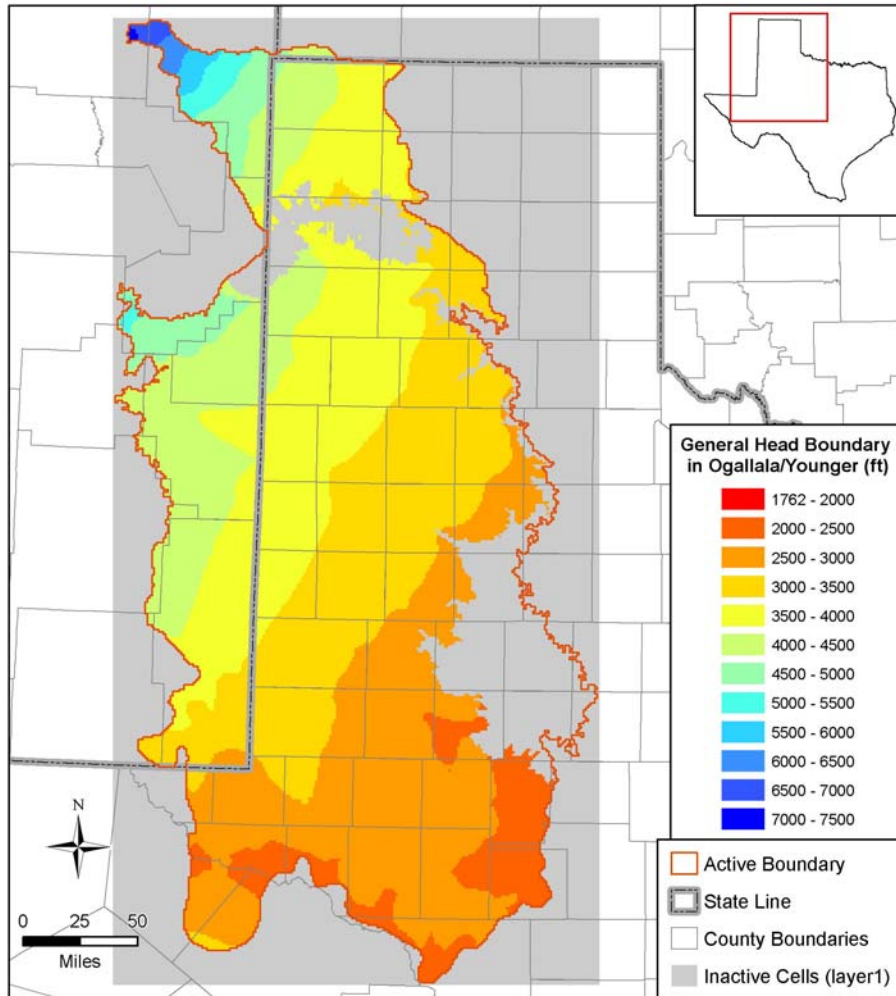


General Head Boundary

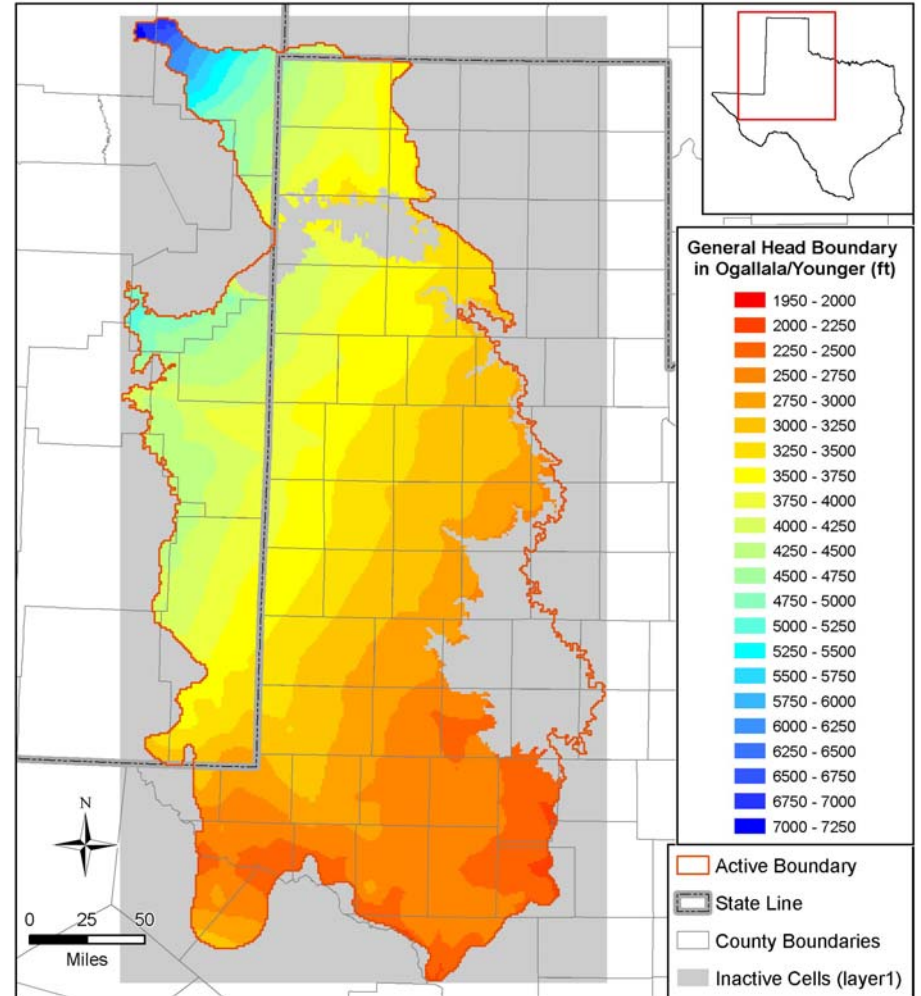
- Used to represent impact of the overlying aquifers
- Pre-development heads based on observed data
- Transient heads based on drawdown from Southern Ogallala GAM simulations
- GHB Conductances large → Dockum properties are primary limiter to flow

General Head Boundary

Predevelopment

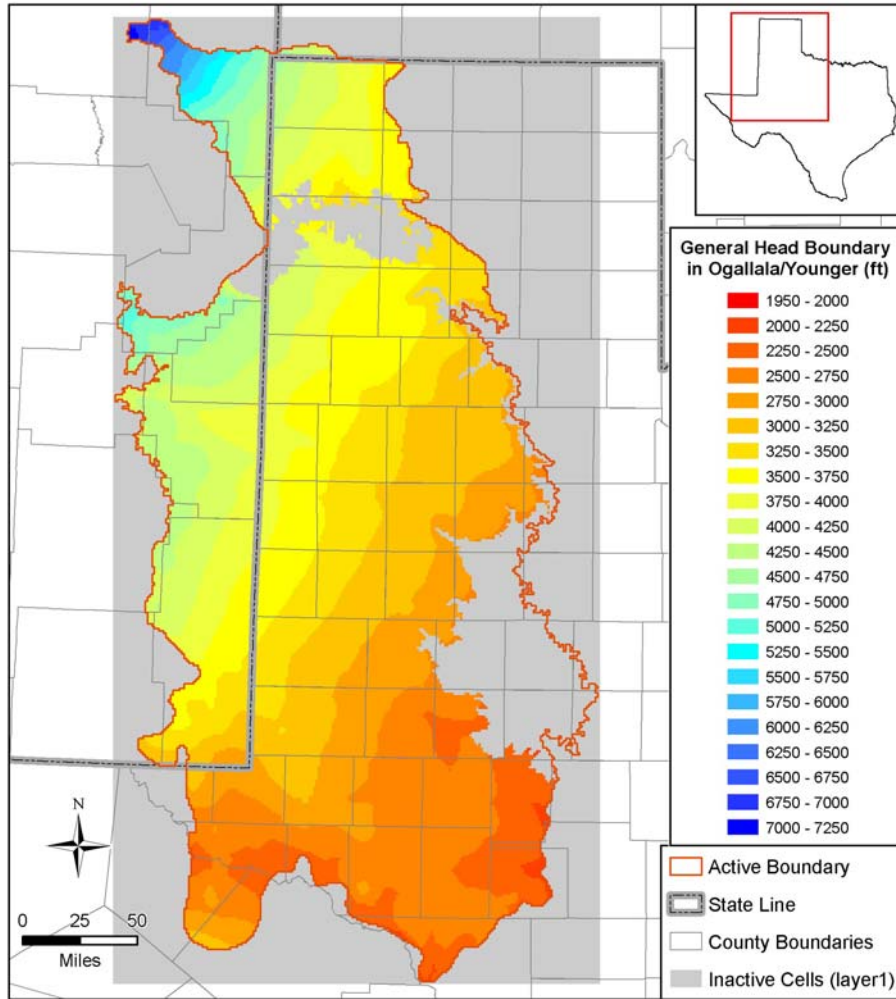


1980

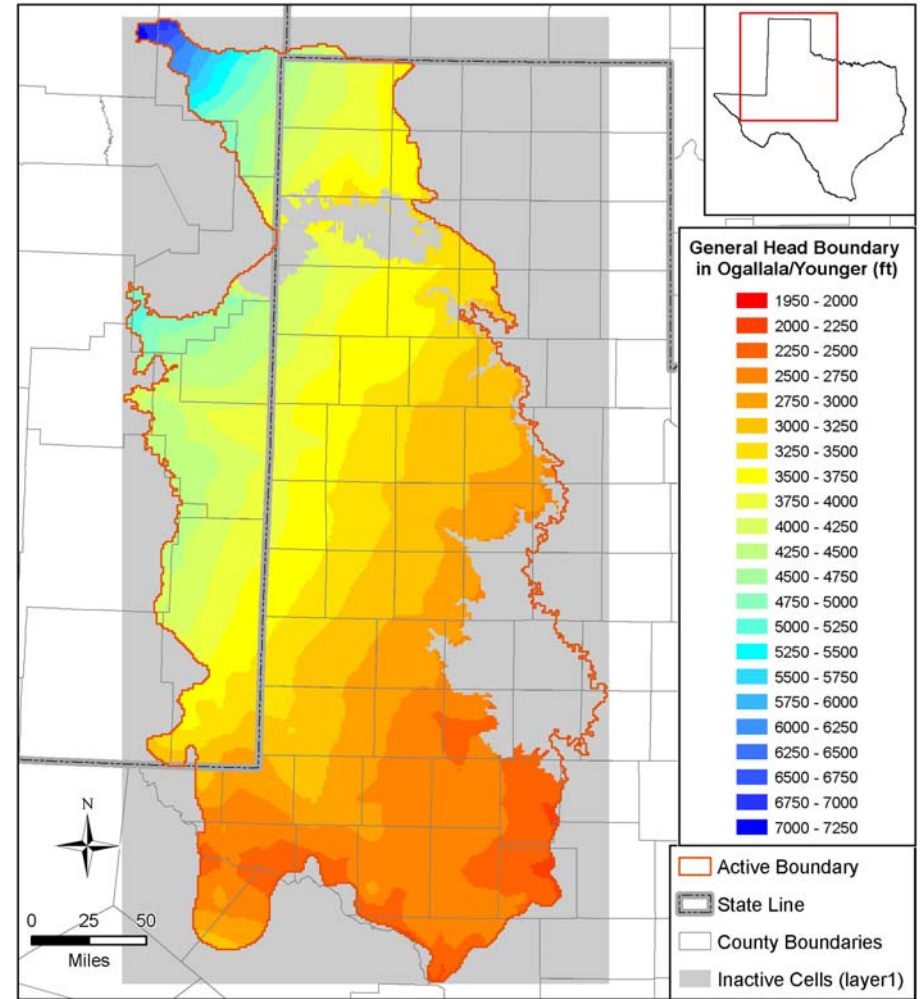


General Head Boundary

1990



1997



Surface Water Boundary Conditions

■ Streams represented by STR package

- Stream geometry (length, slope, width) from RF1 dataset
- Streamflow from RF1 mean flow
- Streambed conductance calibration parameter

■ Springs represented by DRN package

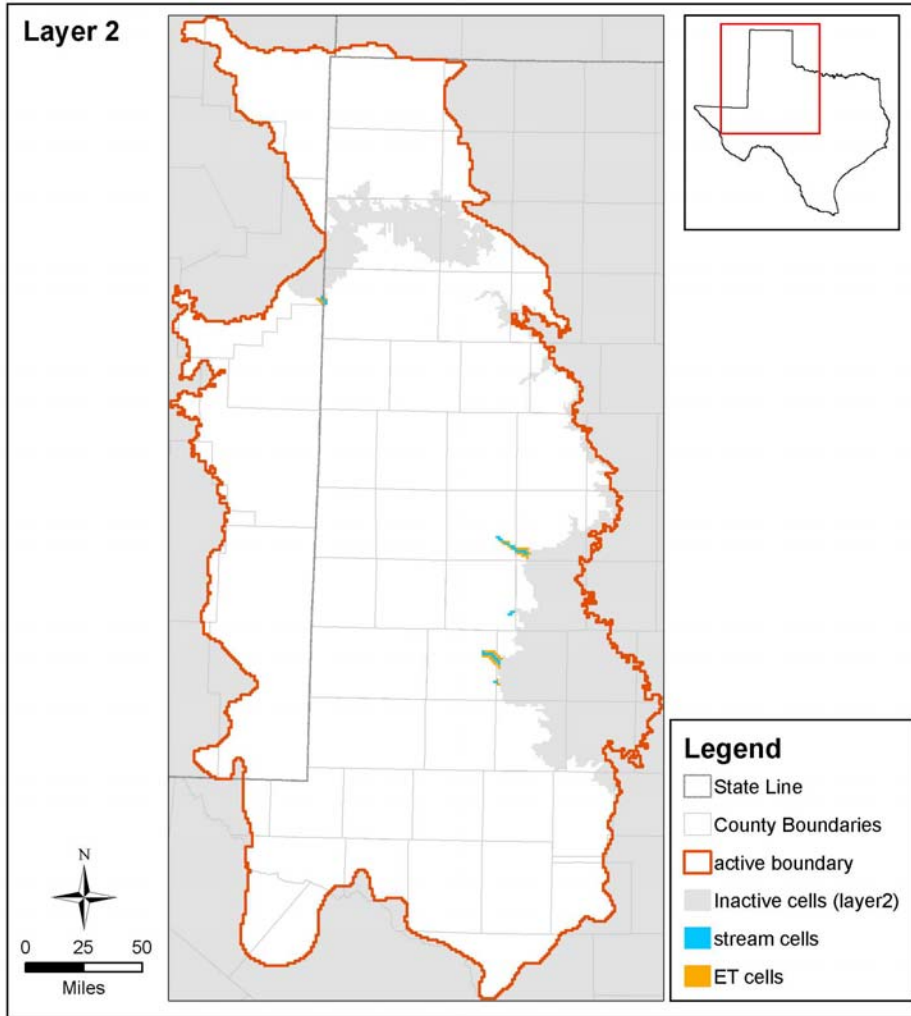
- Drain elevations based on literature
- Conductance calibration parameter

■ Evapotranspiration represented by DRN package

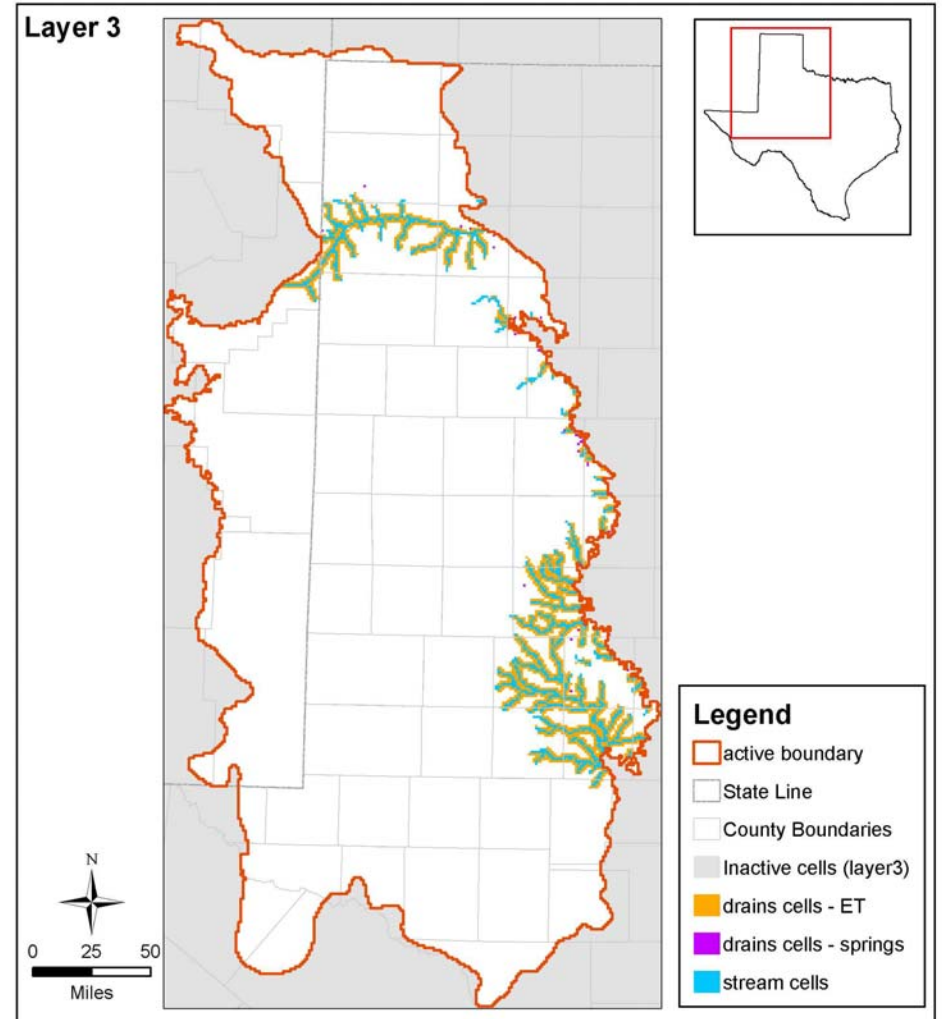
- Attempt to use EVT package following Scanlon et al. 2005 resulted in convergence problems associated with EVT package
- Drain elevation set to root extinction depth
- Conductance large and flows compared with ET_{\max}

Surface Water Boundary Conditions

Upper Dockum



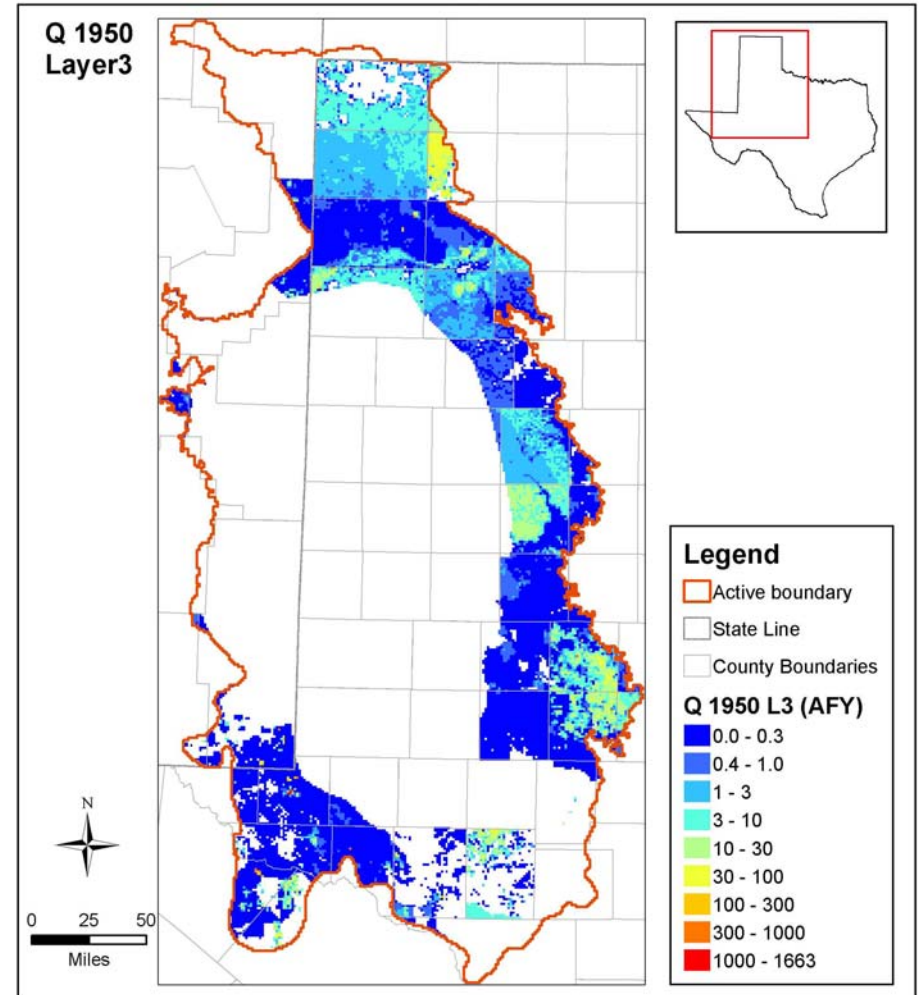
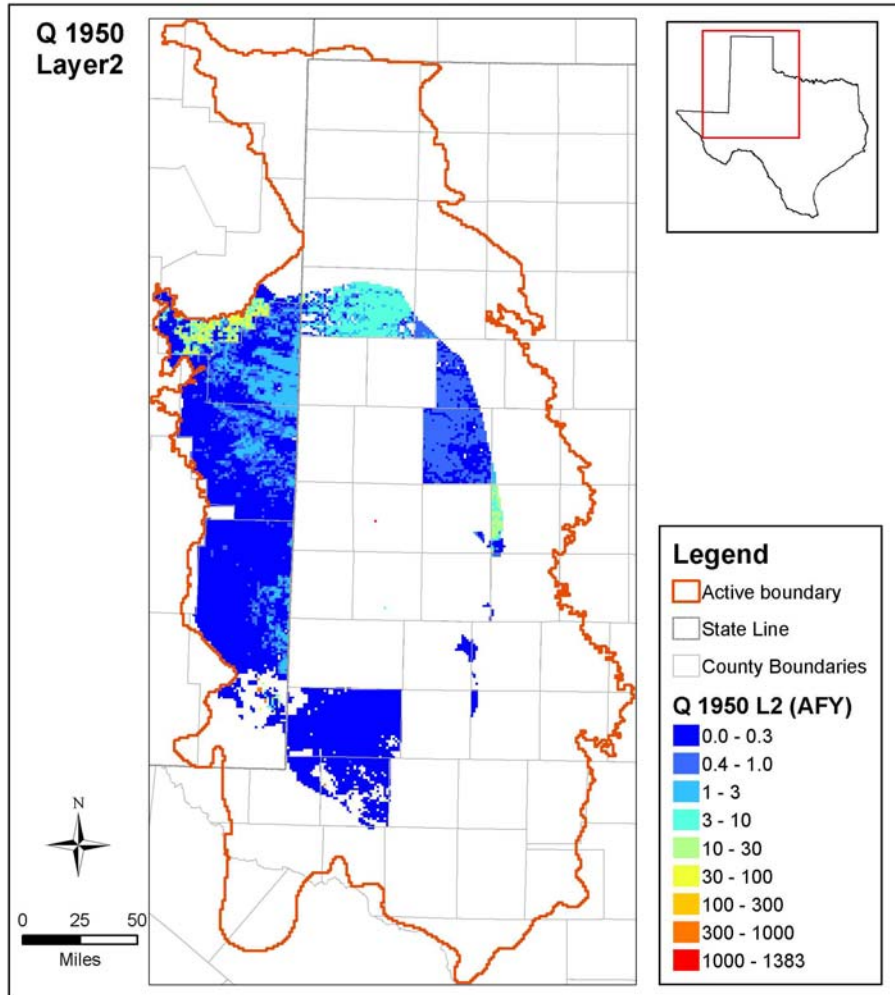
Lower Dockum



Pumping – 1950

Upper Dockum

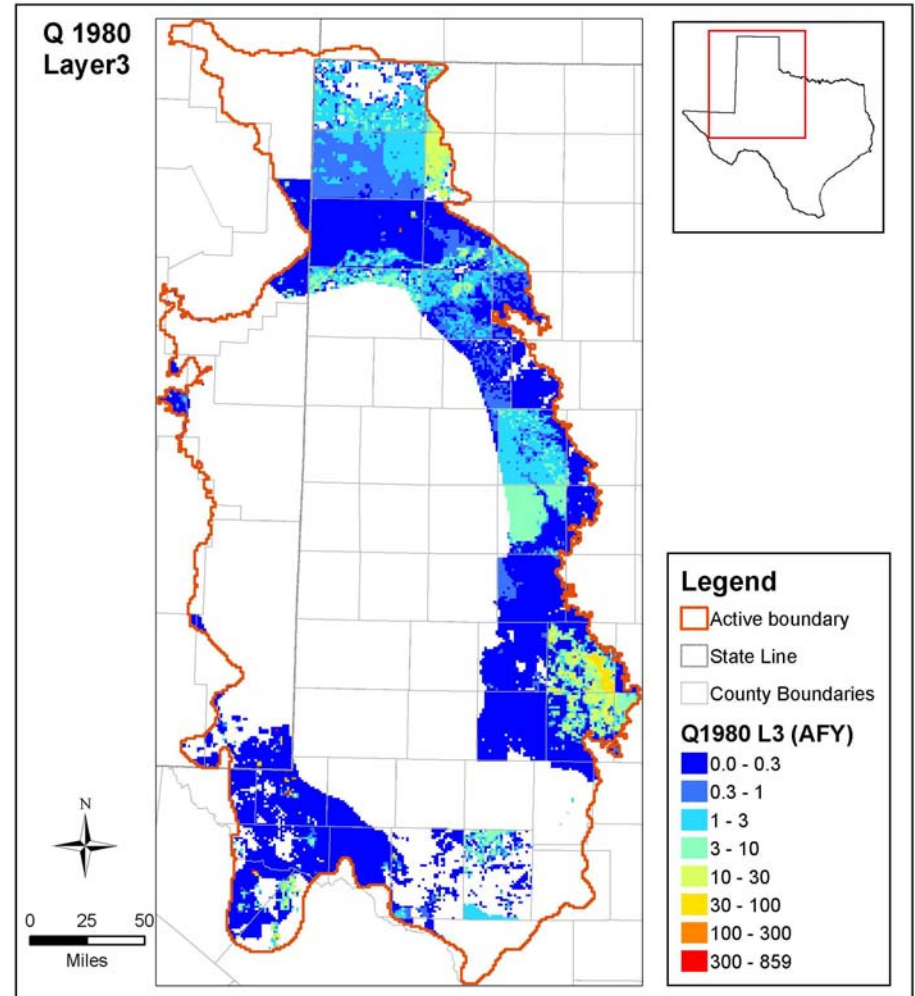
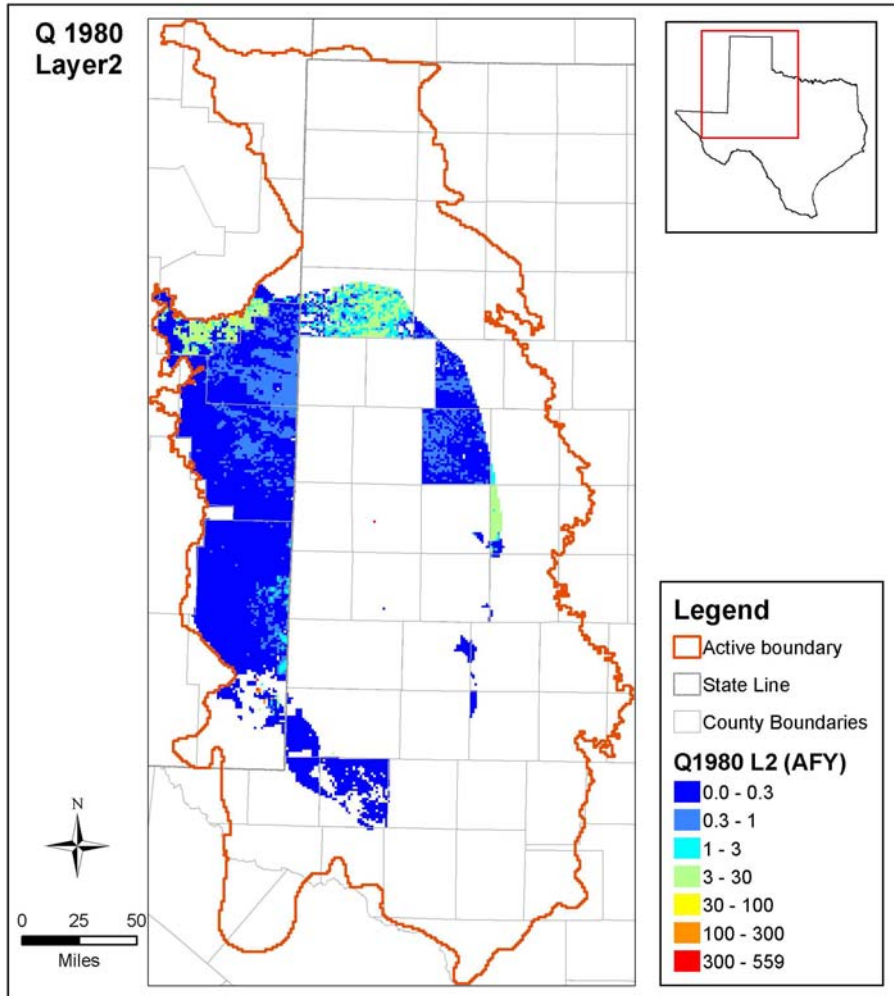
Lower Dockum



Pumping – 1980

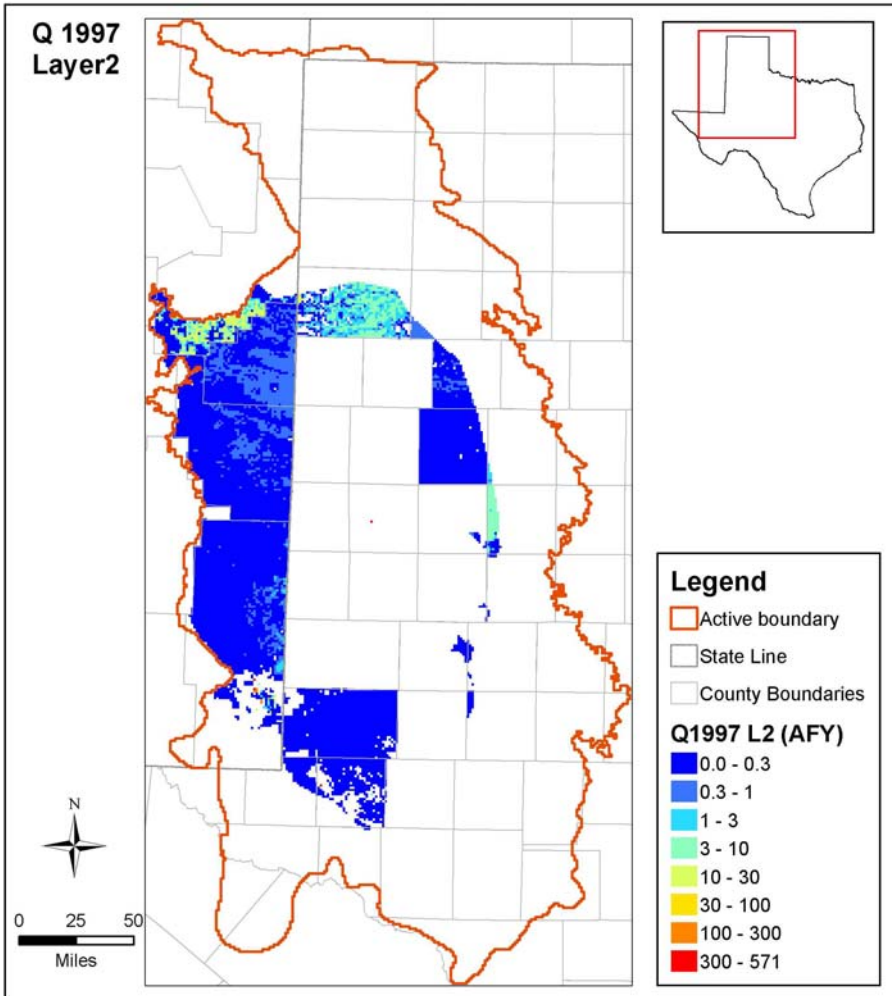
Upper Dockum

Lower Dockum

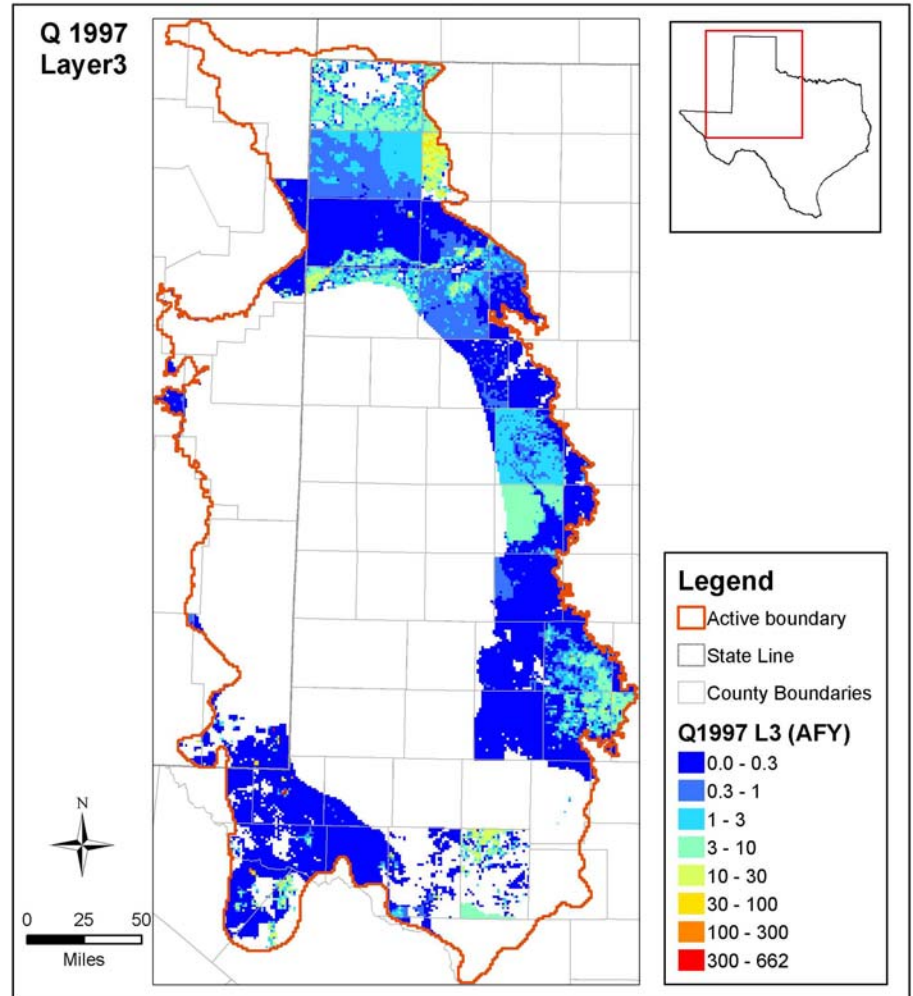


Pumping – 1997

Upper Dockum



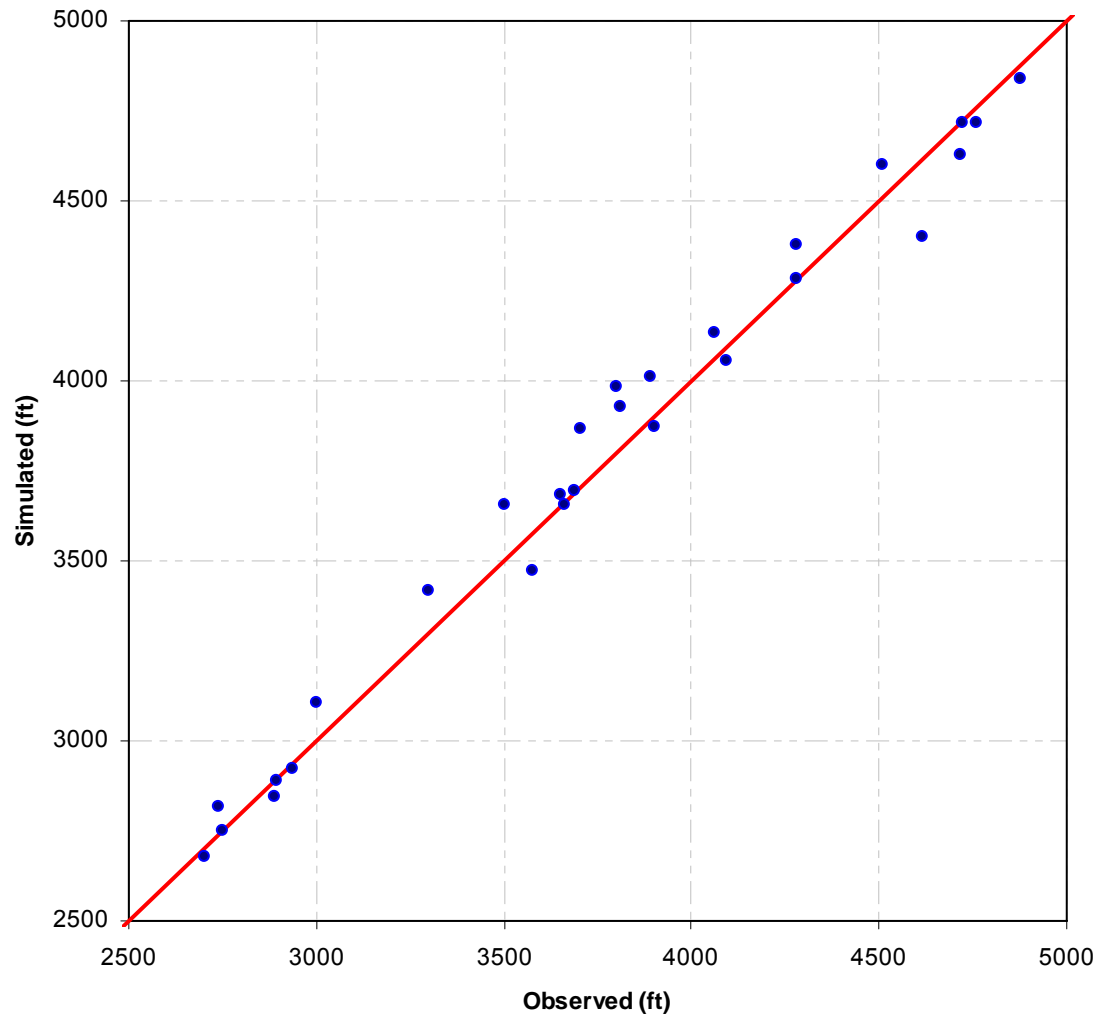
Lower Dockum



Model Calibration

Upper Dockum Calibration – Predevelopment

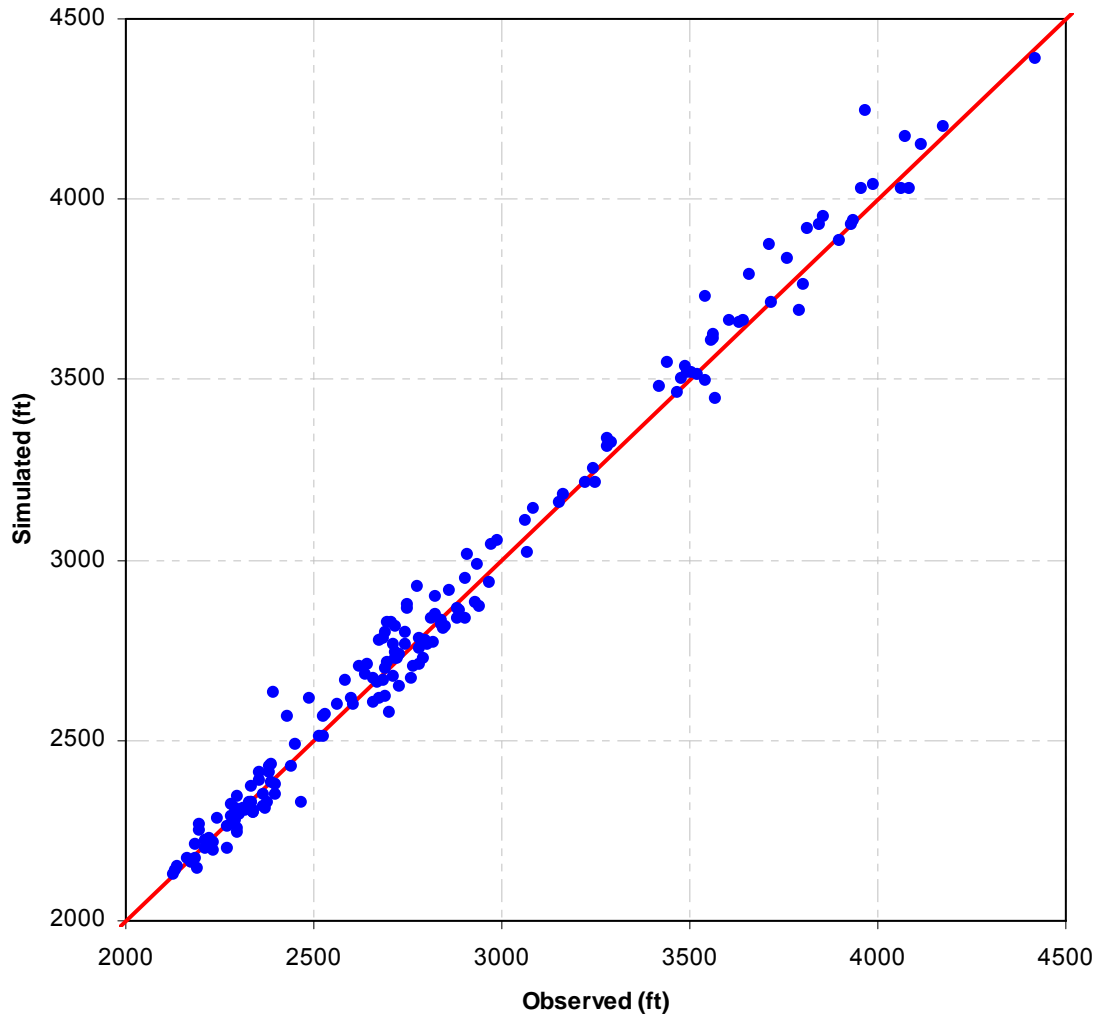
Steady-State Upper Dockum Heads



| | |
|------------|-----------|
| RMSE | 100.0 ft |
| MAE | 77.0 ft |
| ME | 15.0 ft |
| Min E | -223.4 ft |
| Max E | 185.6 ft |
| Range | 2403.6 ft |
| RMSE/Range | 4.16% |
| MAE/Range | 3.20% |
| ME/Range | 0.62% |

Lower Dockum Calibration – Predevelopment

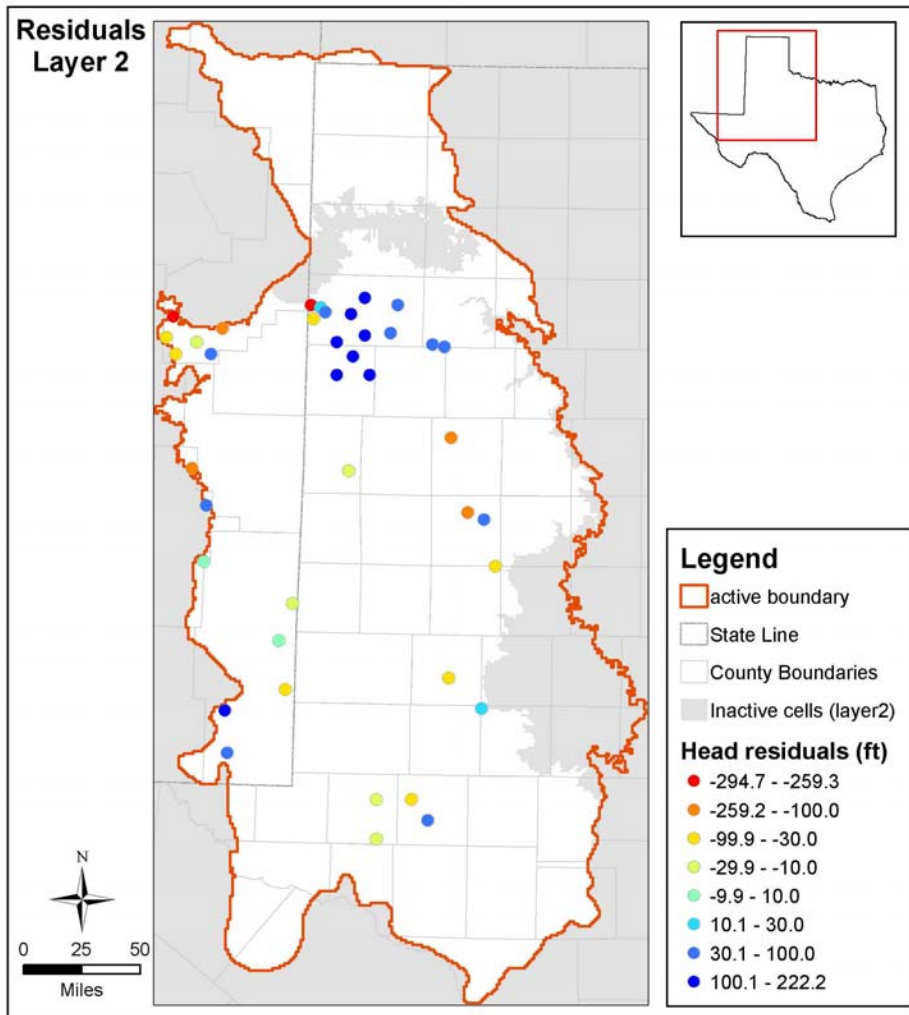
Steady-State Lower Dockum Heads



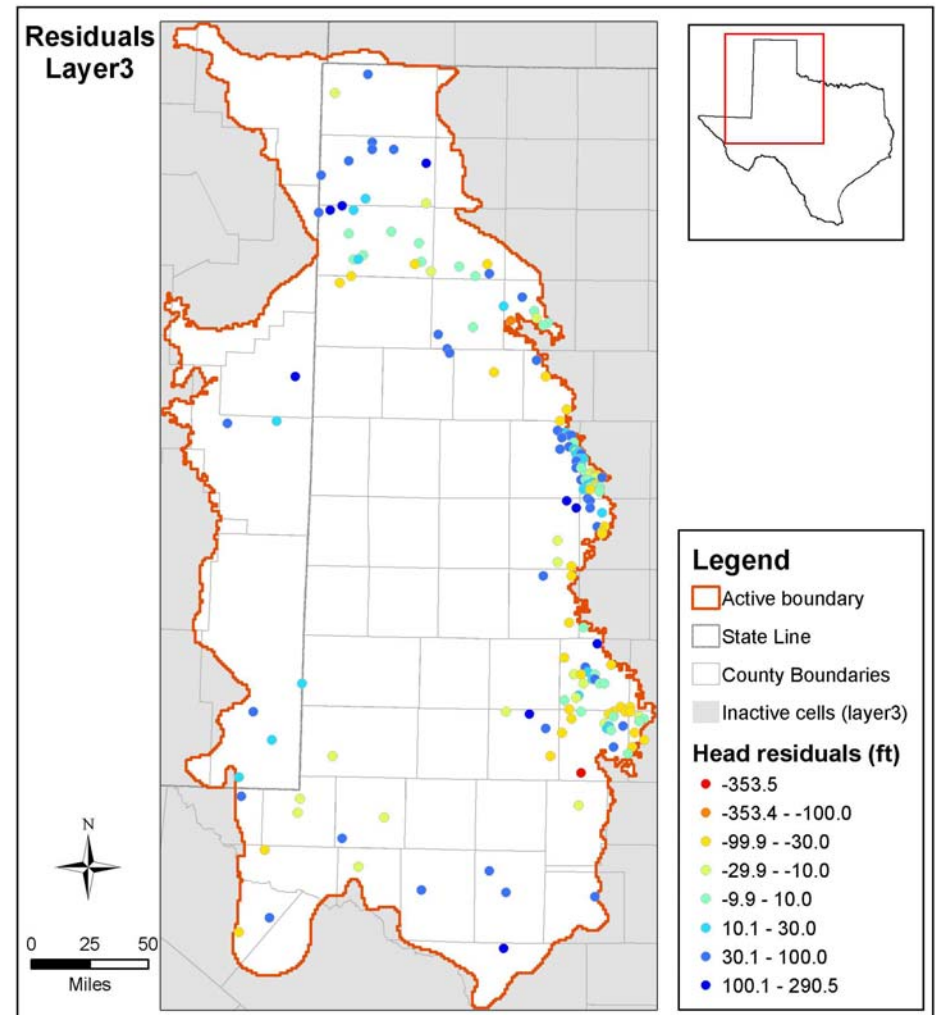
| | |
|------------|-----------|
| RMSE | 65.1 ft |
| MAE | 48.4 ft |
| ME | 15.1 ft |
| Min E | -136.7 ft |
| Max E | 273.8 ft |
| Range | 2288.5 ft |
| RMSE/Range | 2.84% |
| MAE/Range | 2.12% |
| ME/Range | 0.66% |

Predevelopment Head Residuals

Upper Dockum

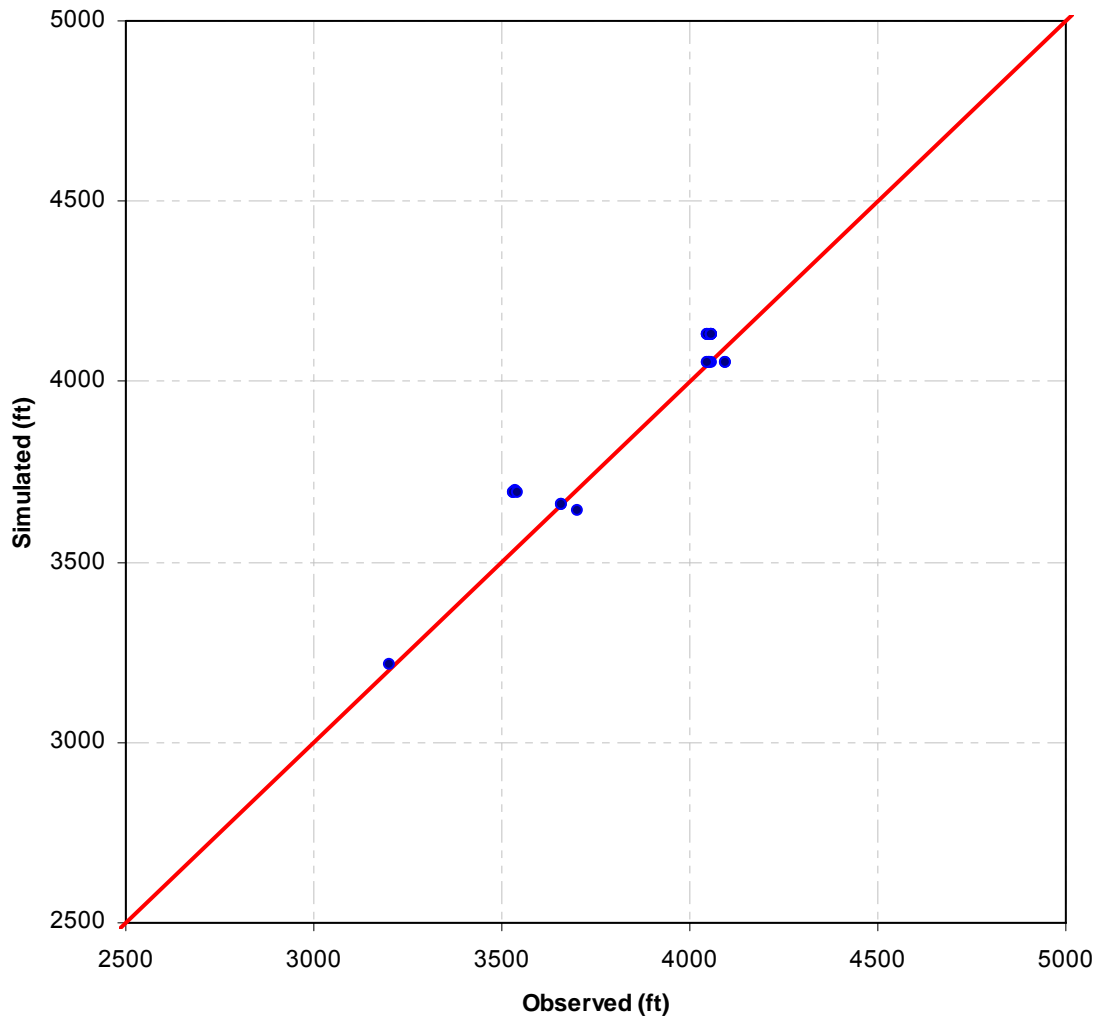


Lower Dockum



Upper Dockum Calibration – Transient

Transient Upper Dockum Heads

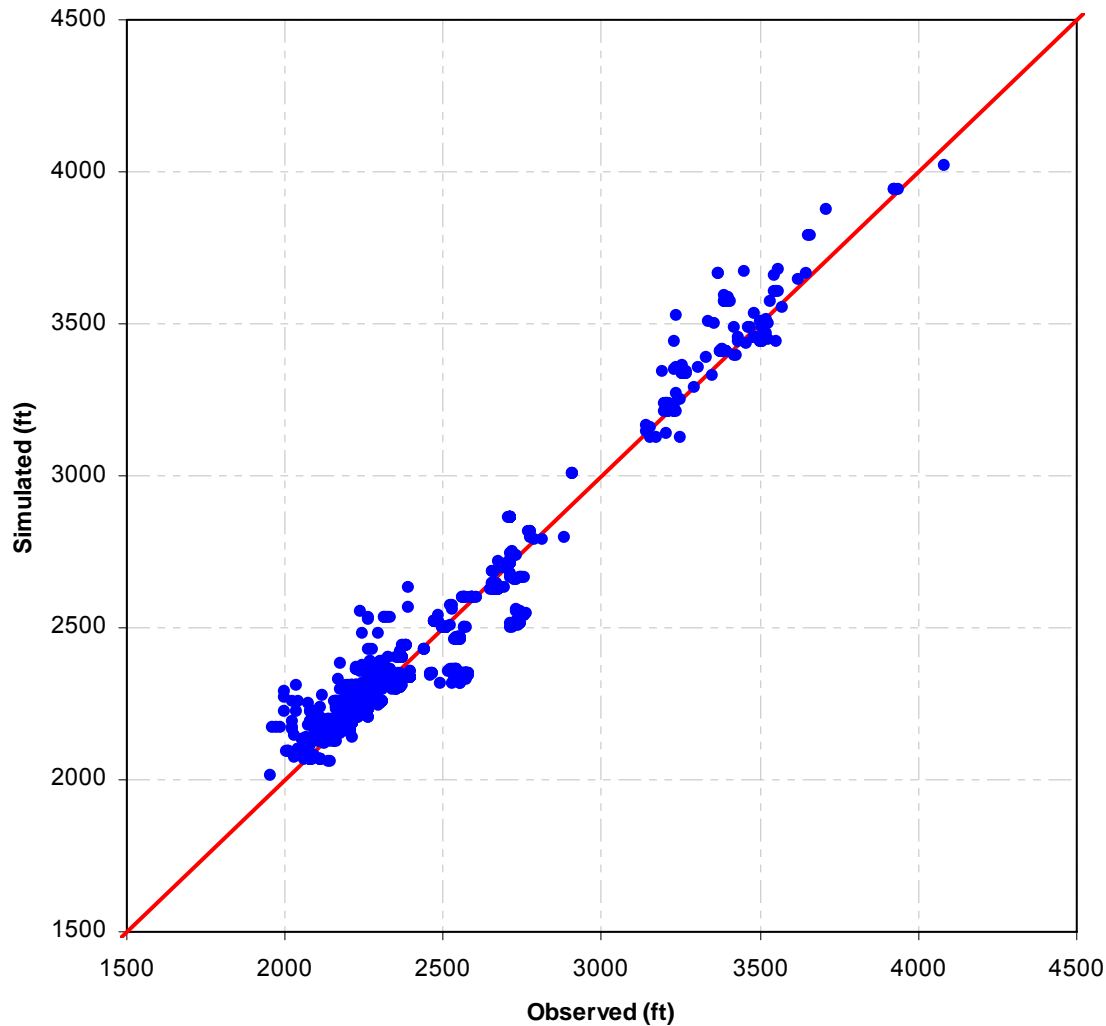


| | |
|-------|----------|
| RMSE | 80.3 ft |
| MAE | 60.3 ft |
| ME | 46.8 ft |
| Min E | -60.3 ft |
| Max E | 164.0 ft |

| | |
|------------|-----------|
| Range | 2403.6 ft |
| RMSE/Range | 3.34% |
| MAE/Range | 2.51% |
| ME/Range | 1.95% |

Lower Dockum Calibration – Transient

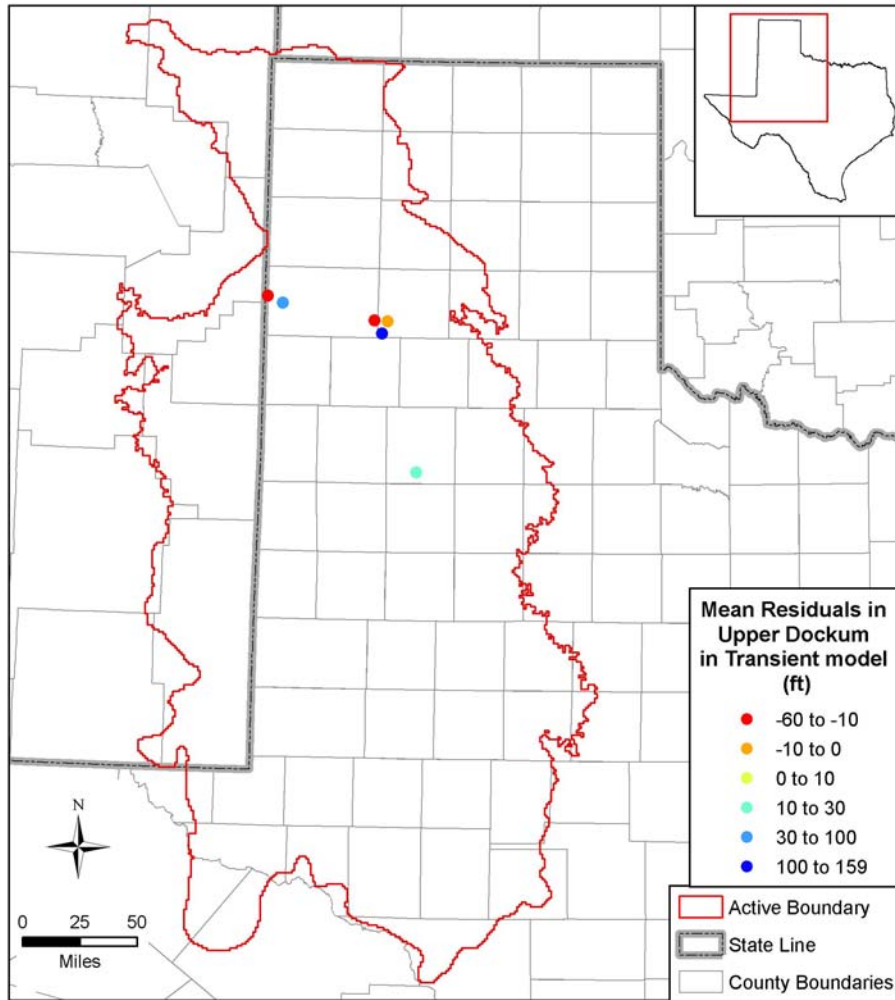
Transient Lower Dockum Heads



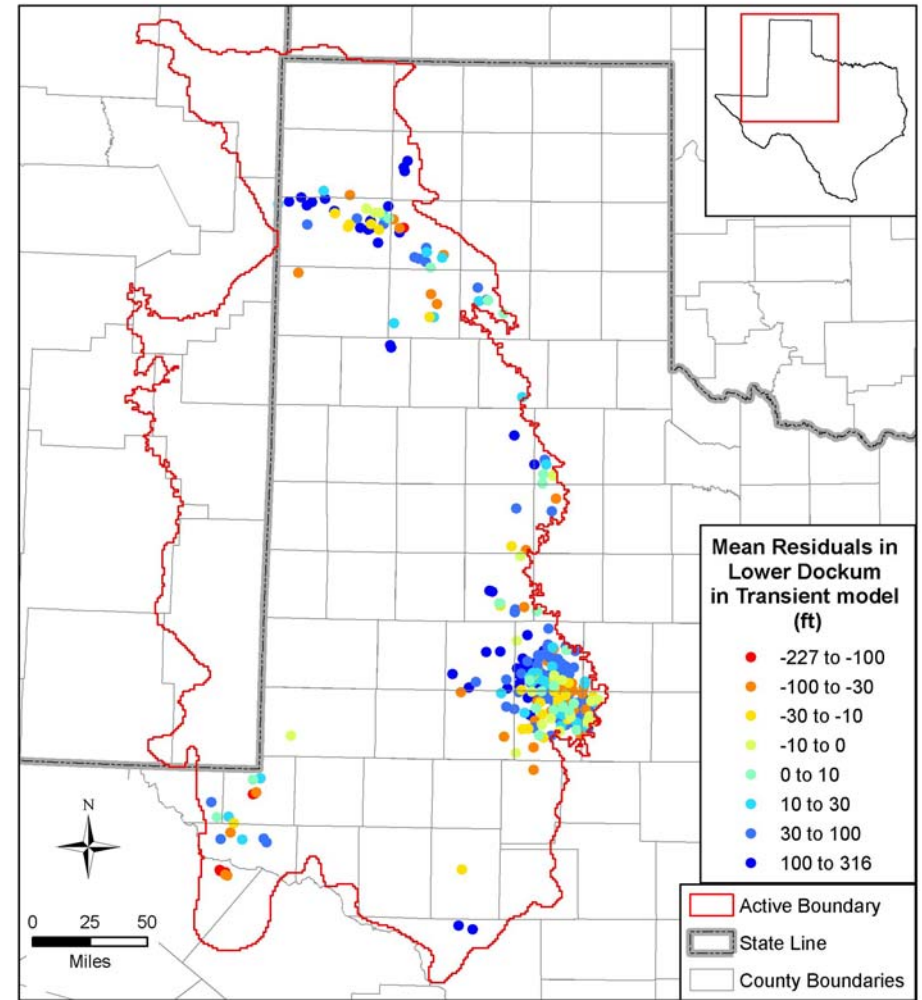
| | |
|------------|-----------|
| RMSE | 94.3 ft |
| MAE | 65.6 ft |
| ME | 0.1 ft |
| Min E | -243.6 ft |
| Max E | 316.2 ft |
| Range | 2288.5 ft |
| RMSE/Range | 4.12% |
| MAE/Range | 2.87% |
| ME/Range | 0.01% |

Transient Mean Residuals in Dockum

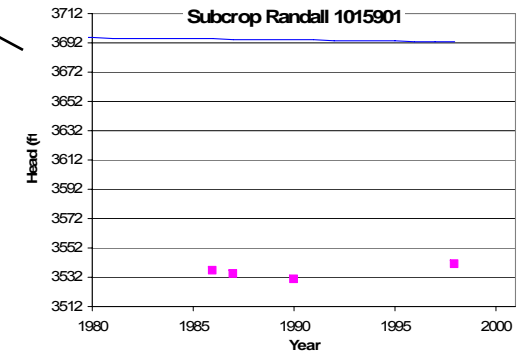
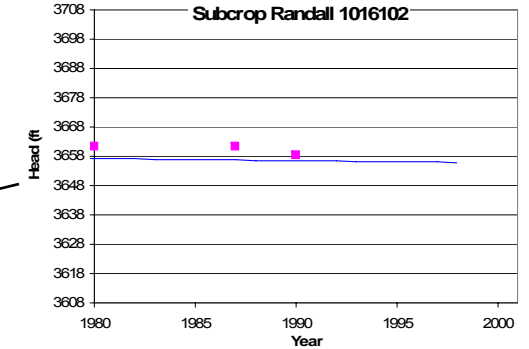
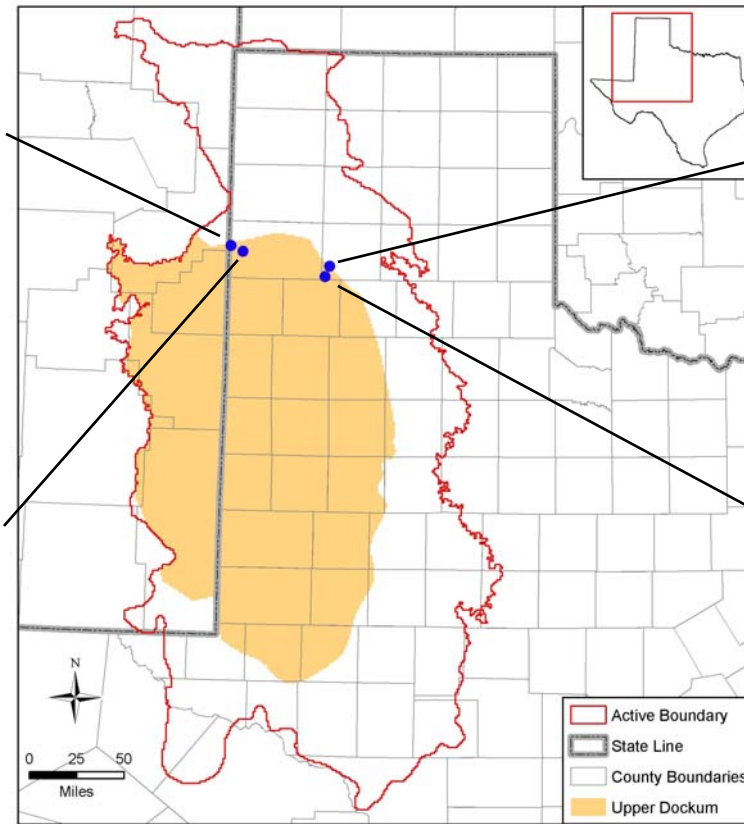
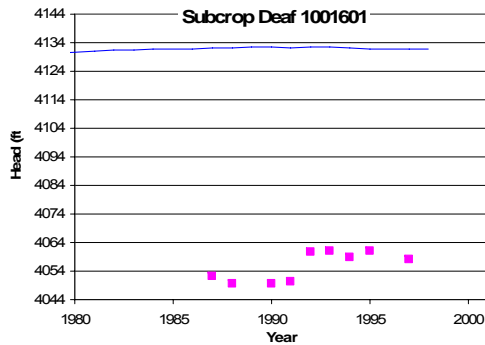
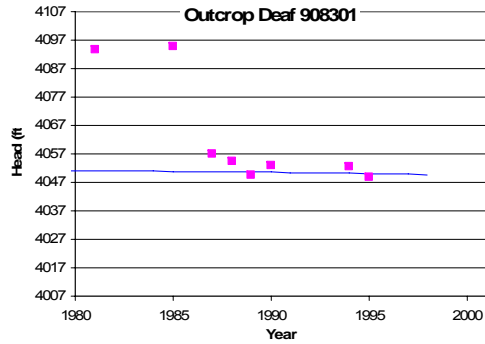
Upper Dockum



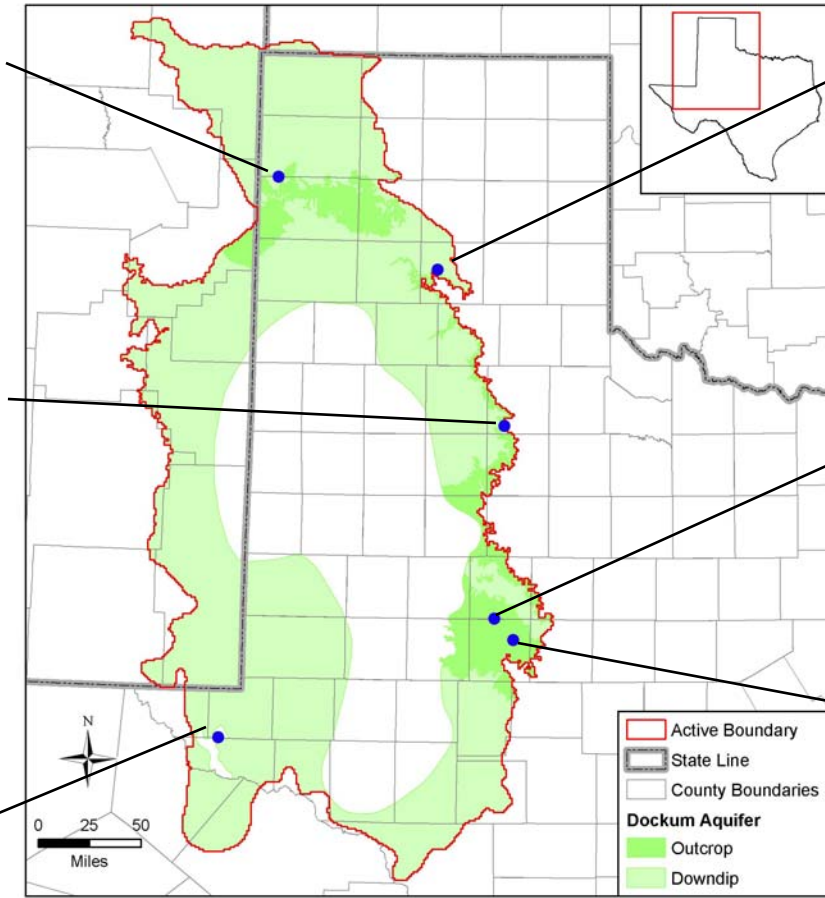
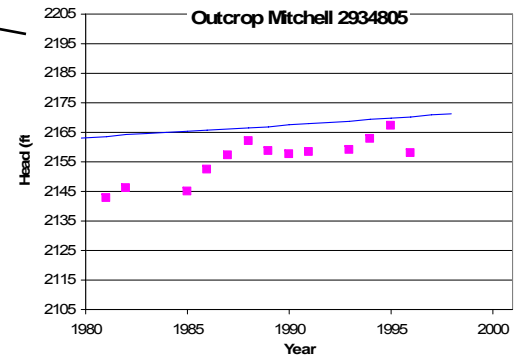
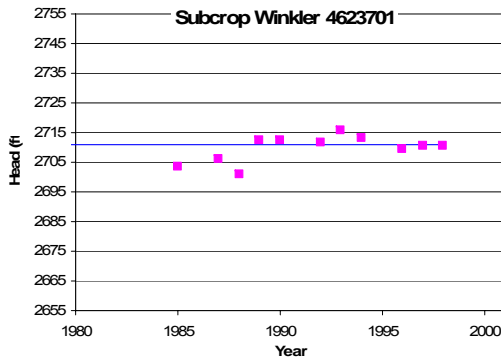
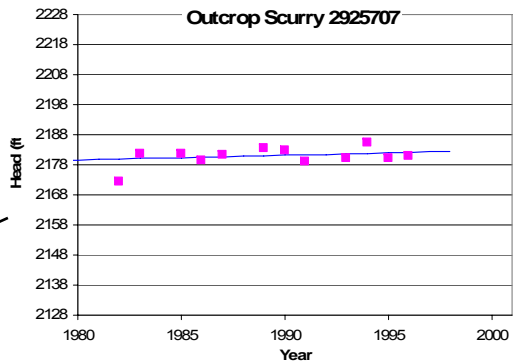
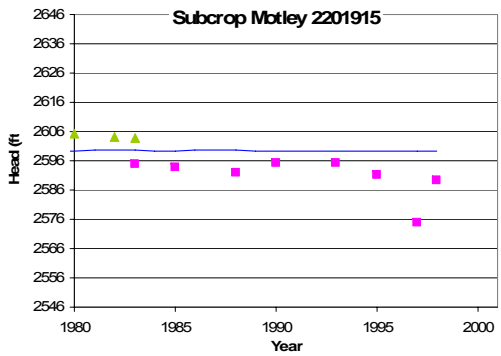
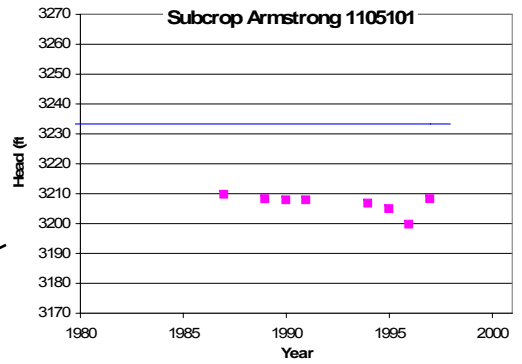
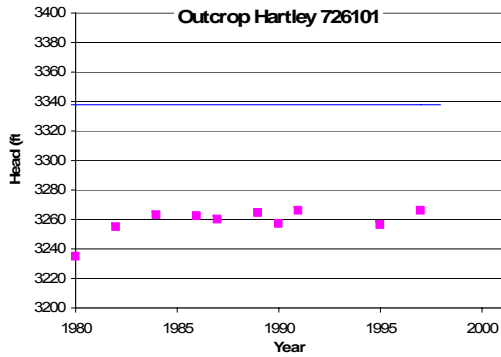
Lower Dockum



Upper Dockum Hydrographs

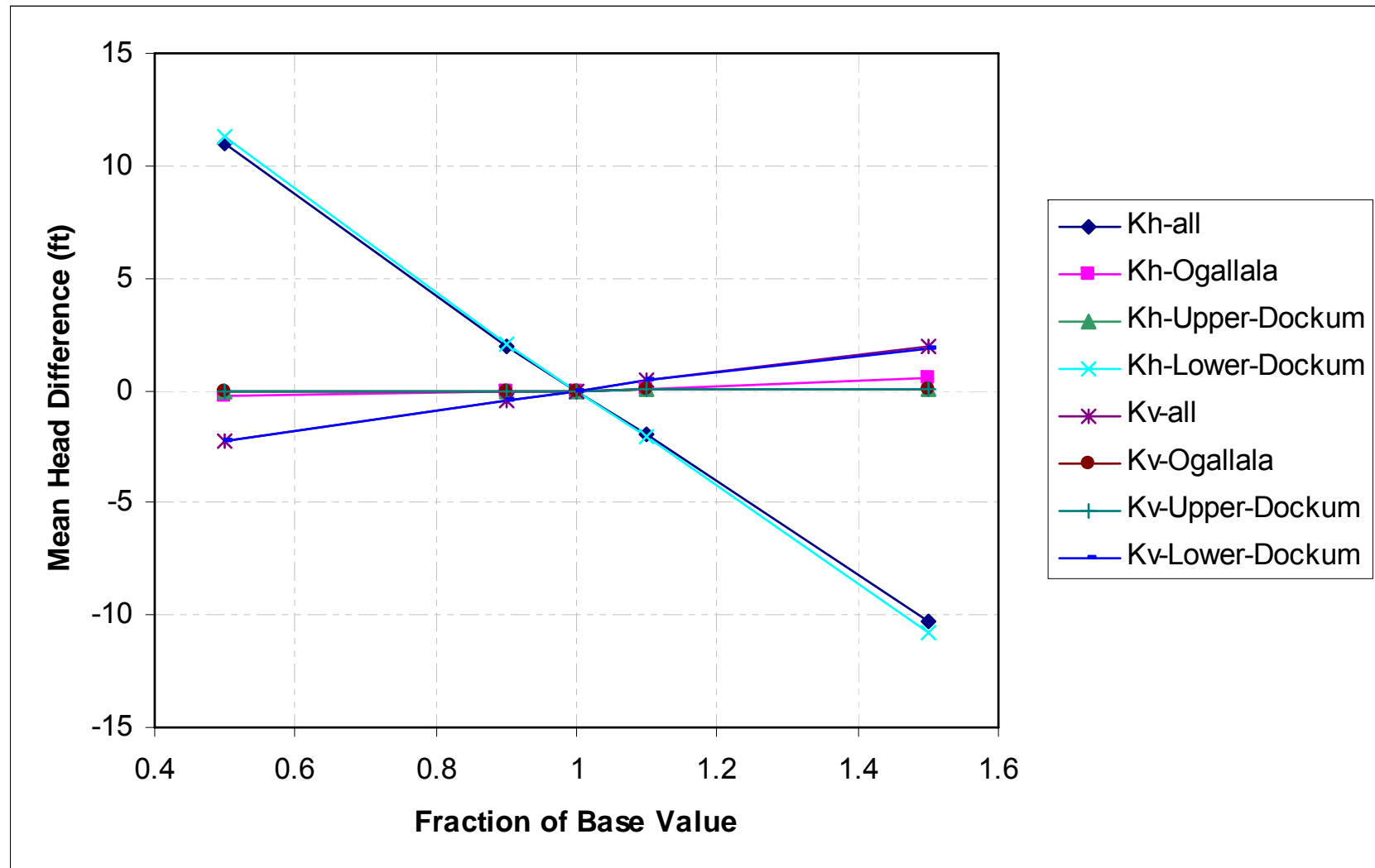


Lower Dockum Hydrographs



Sensitivity Analysis

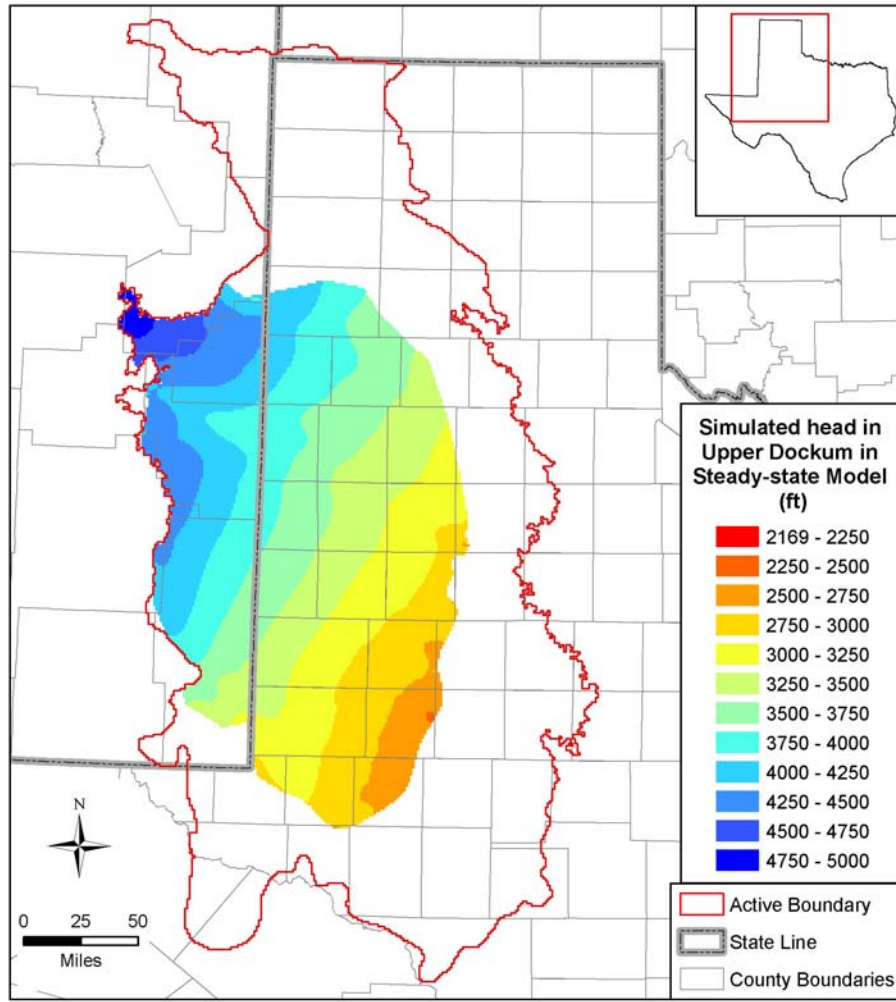
Example: Effect of Hydraulic Parameters on Lower Dockum Heads



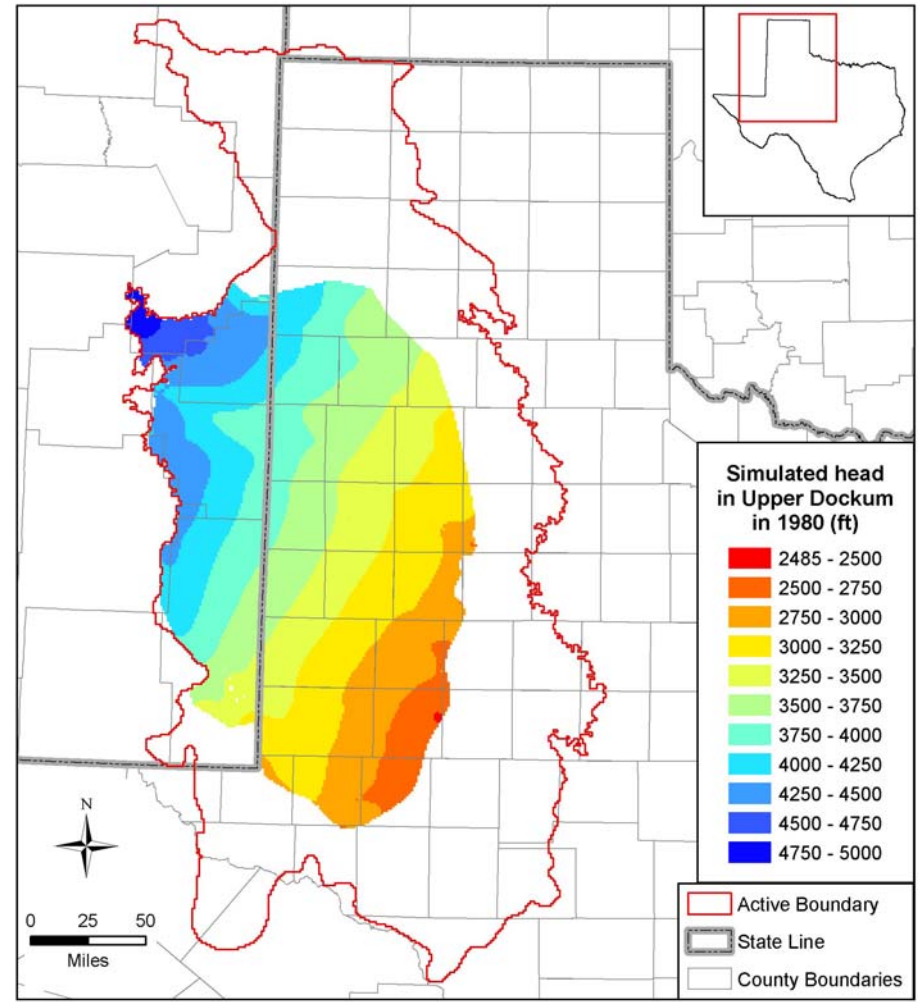
Transient Model Results

Simulated Head in Upper Dockum

Predevelopment

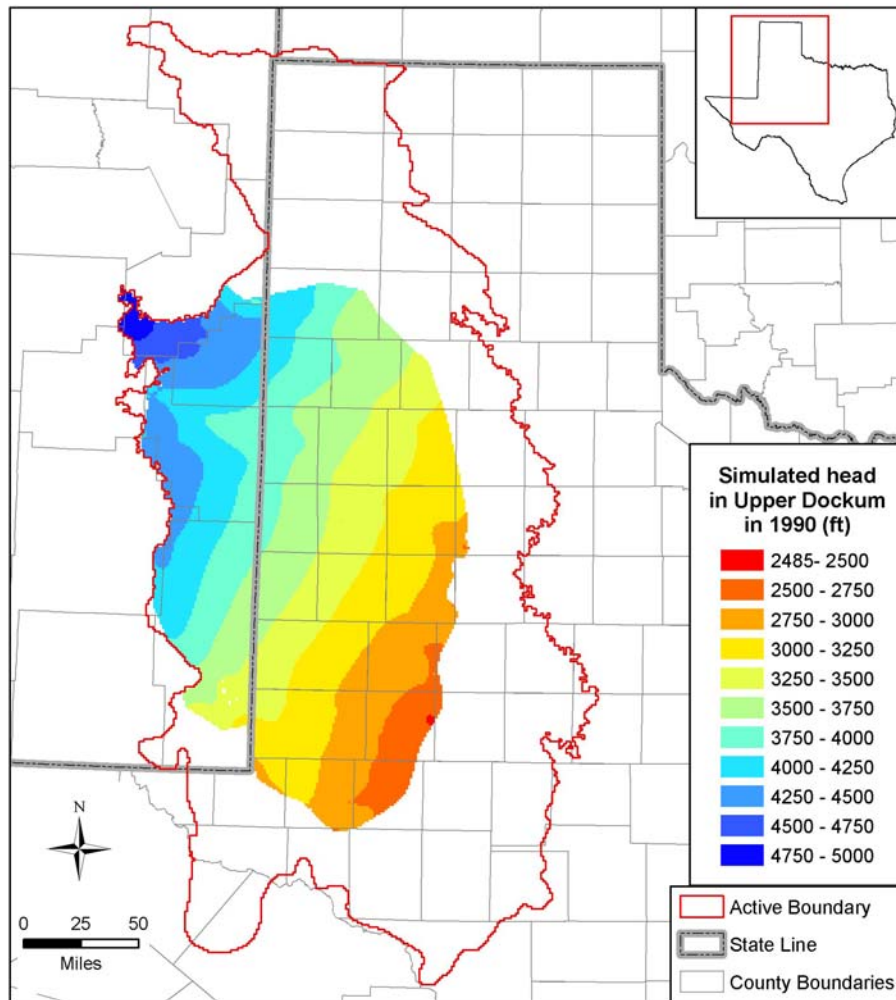


1980

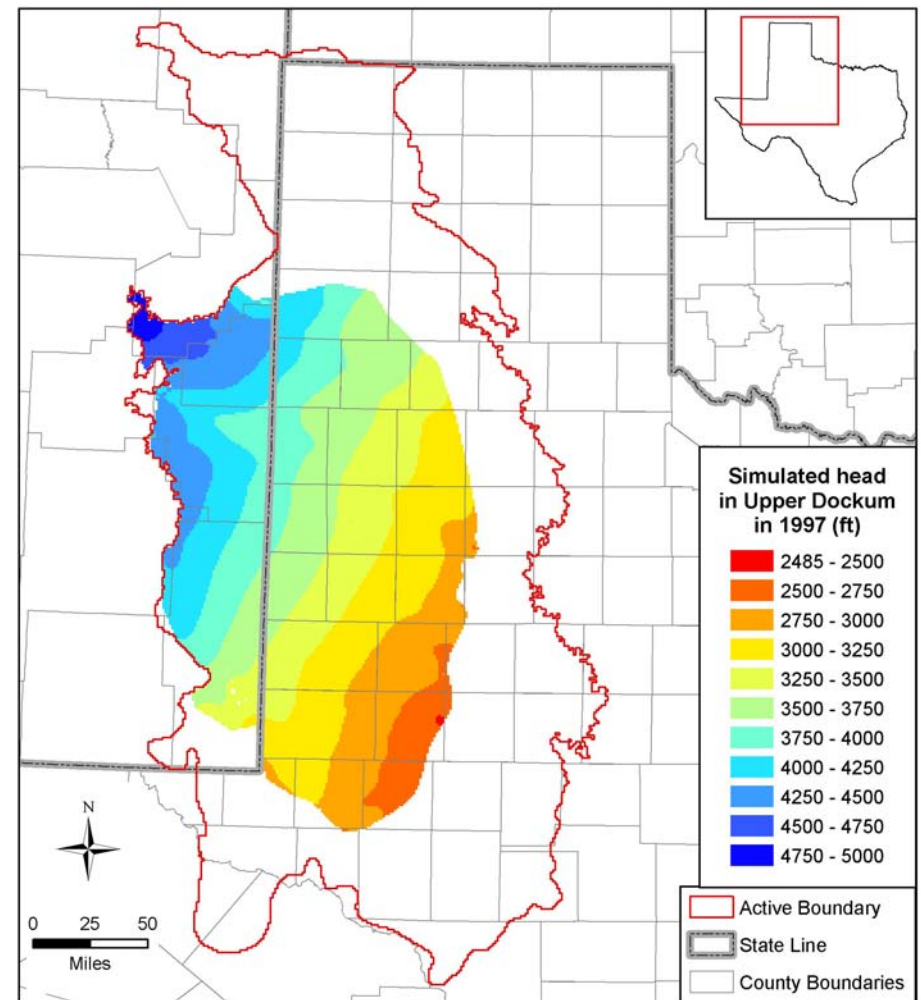


Simulated Head in Upper Dockum

1990

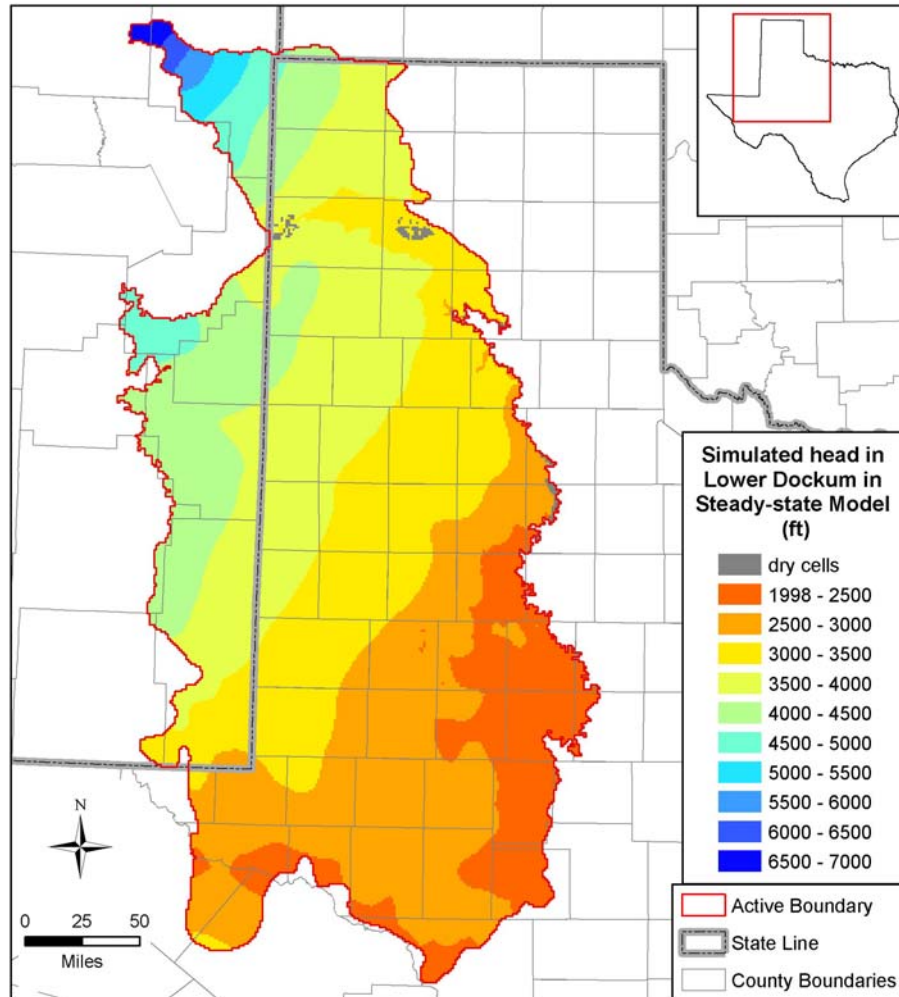


1997

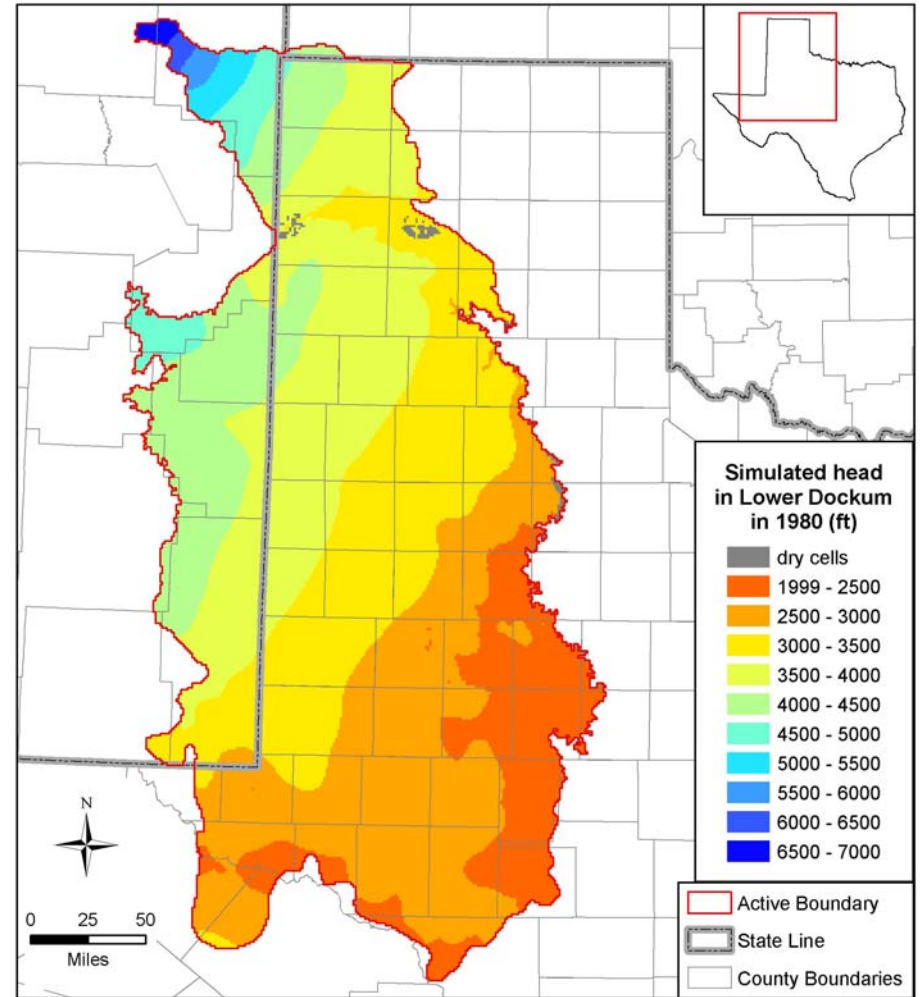


Simulated Head in Lower Dockum

Predevelopment

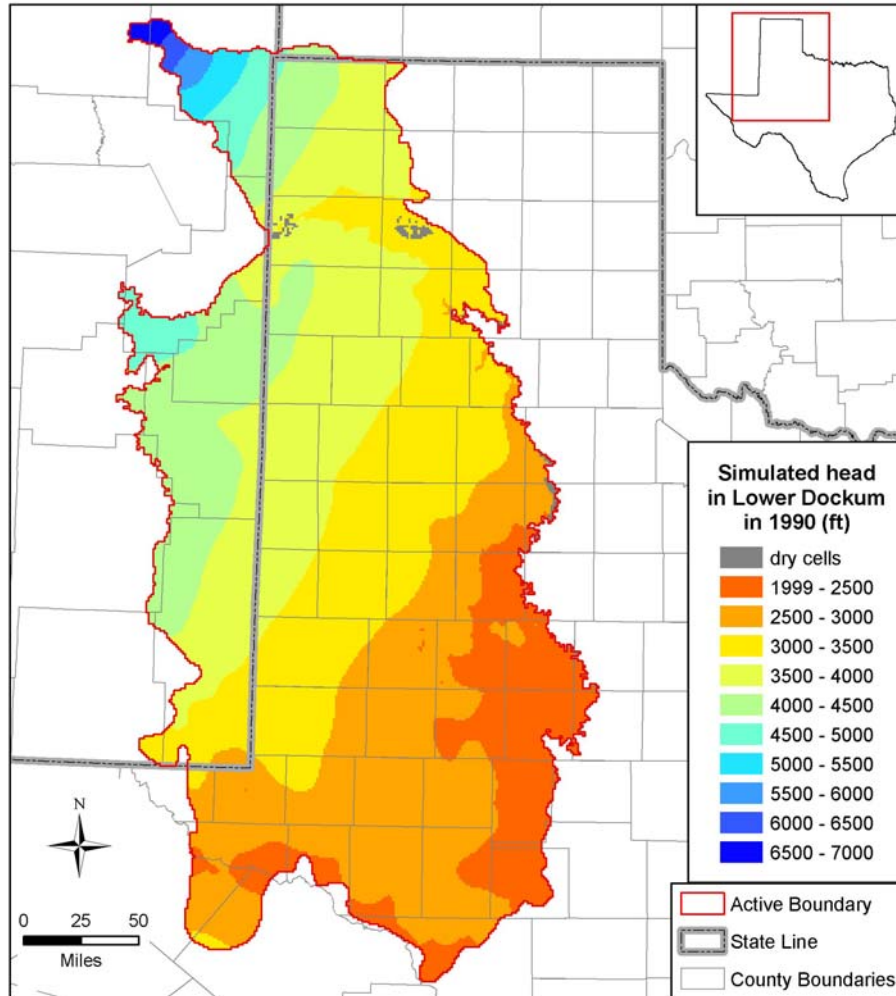


1980

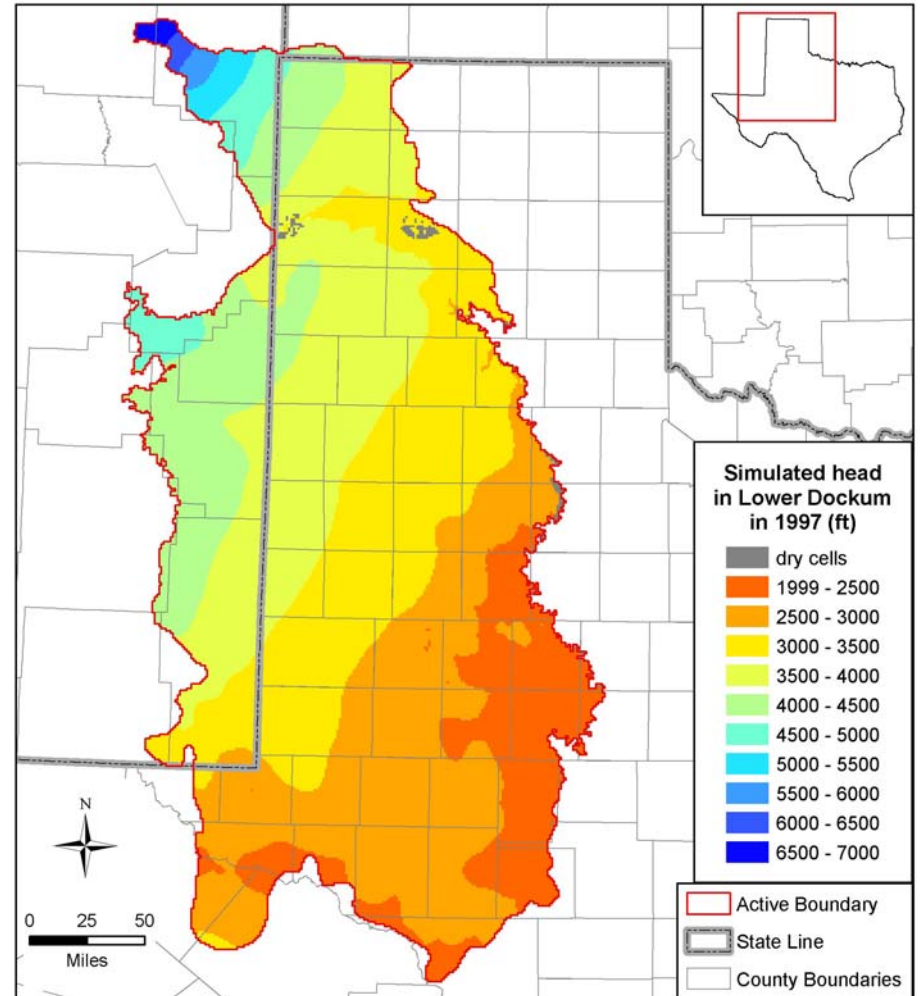


Simulated Head in Lower Dockum

1990

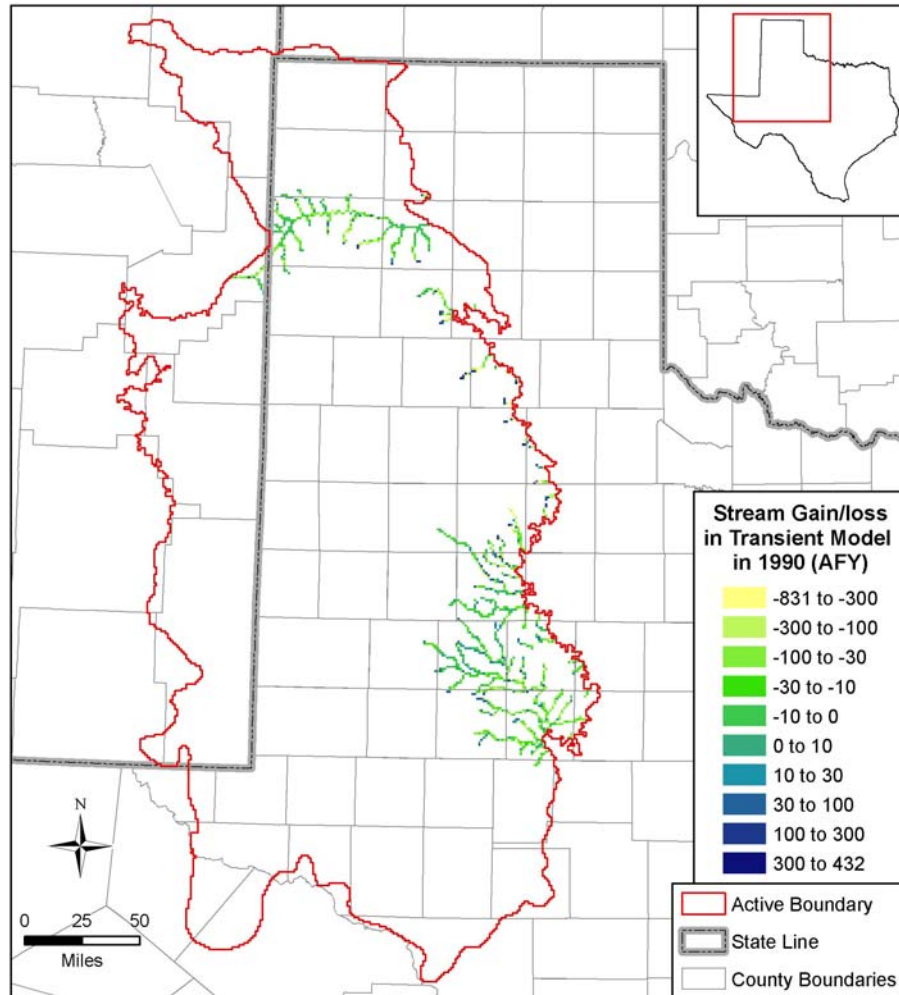


1997

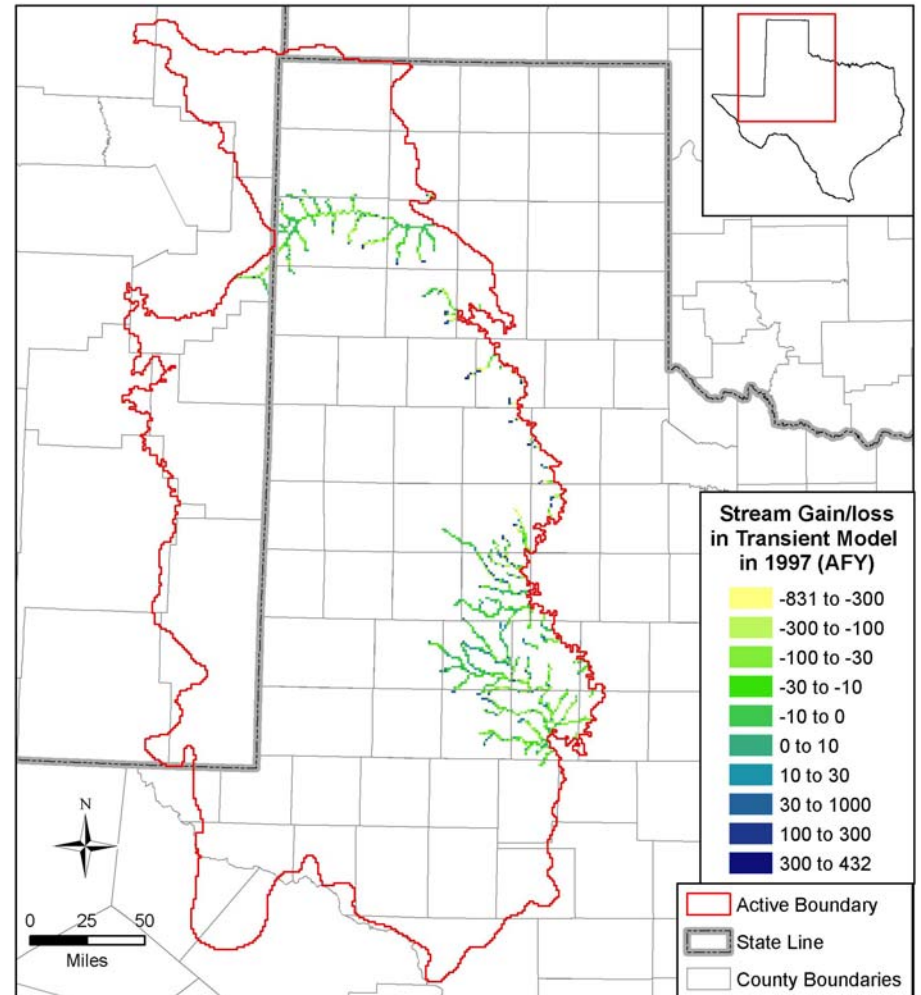


Stream Gain/loss

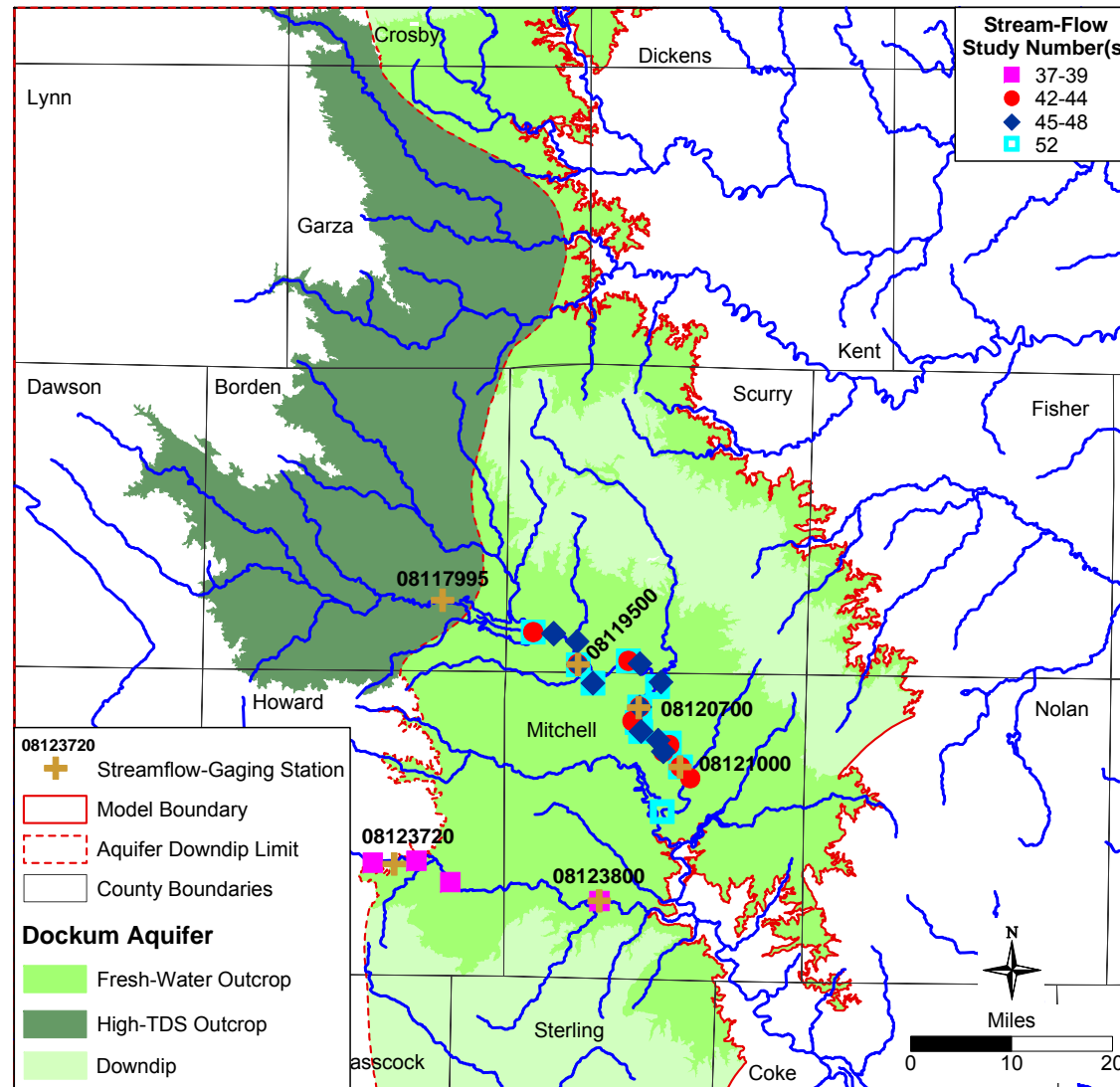
1990



1997

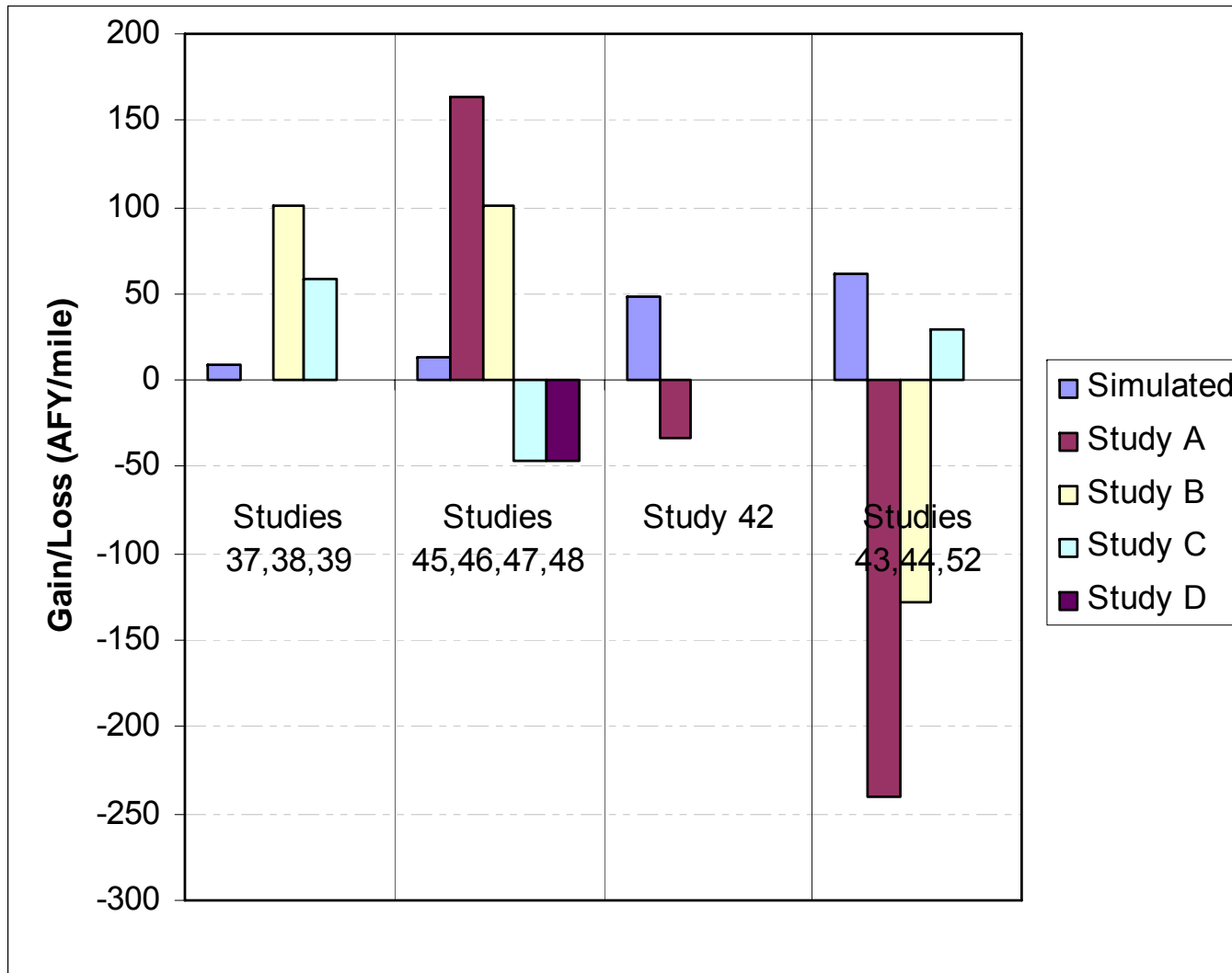


Streams and Rivers – Gain/Loss Studies



Slade et al., (2002)

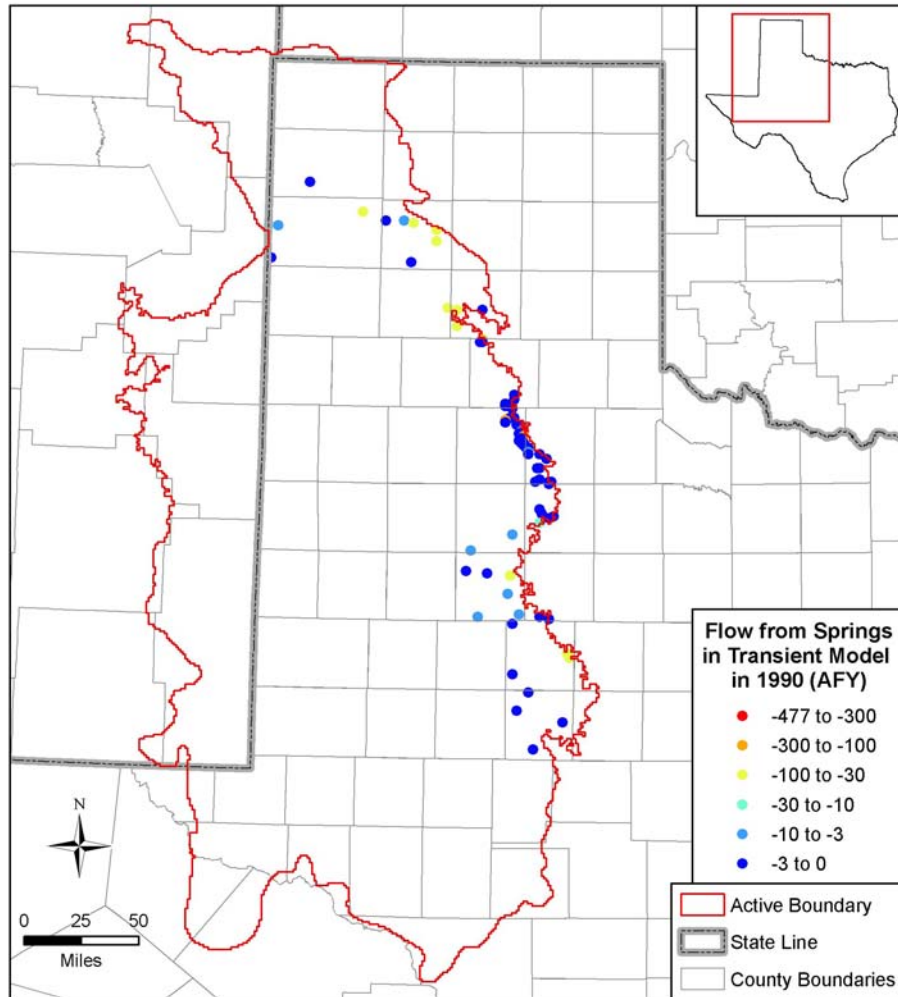
Simulated vs. Observed Stream Gain/Loss



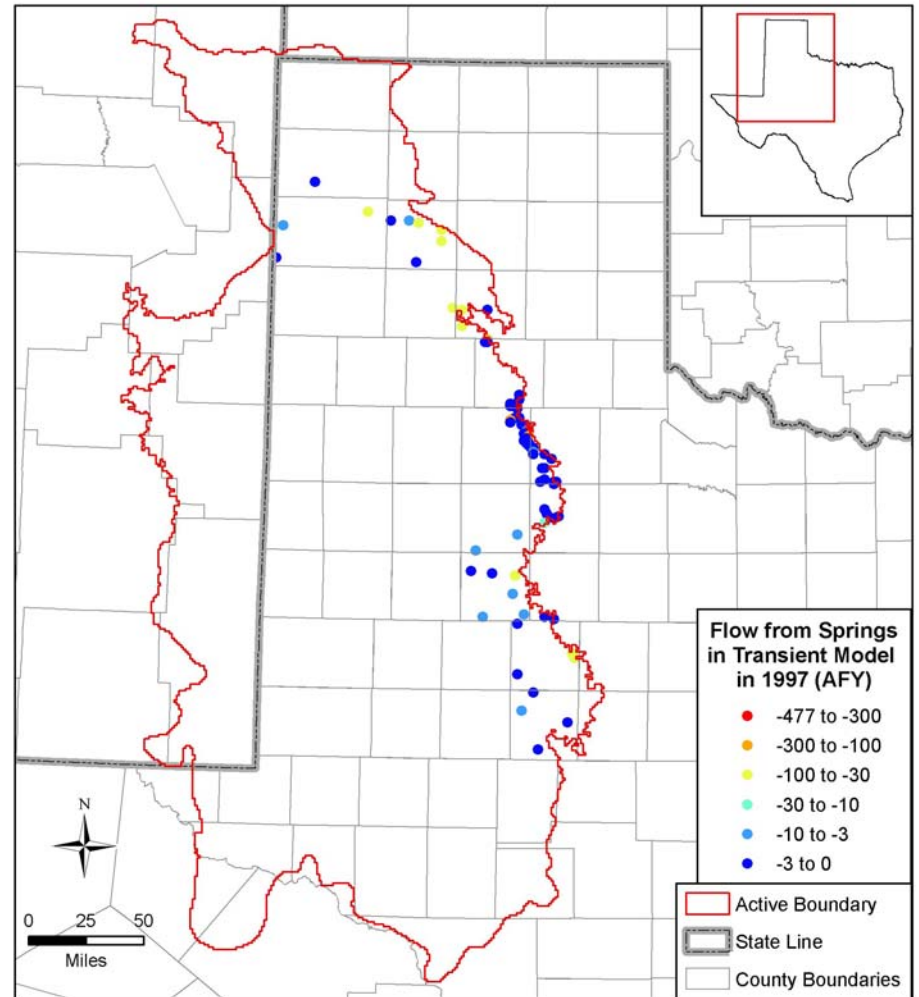
Note: positive denotes gain and negative denotes loss

Spring Flow

1990



1997

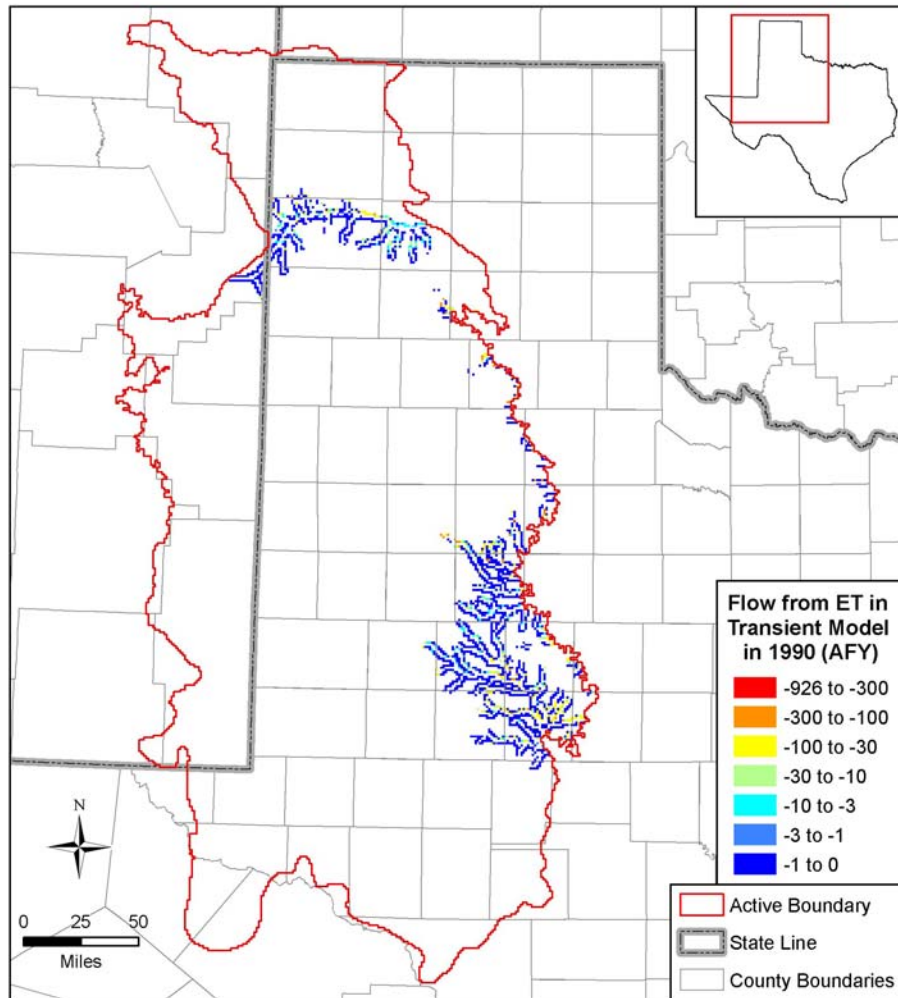


Spring Issues

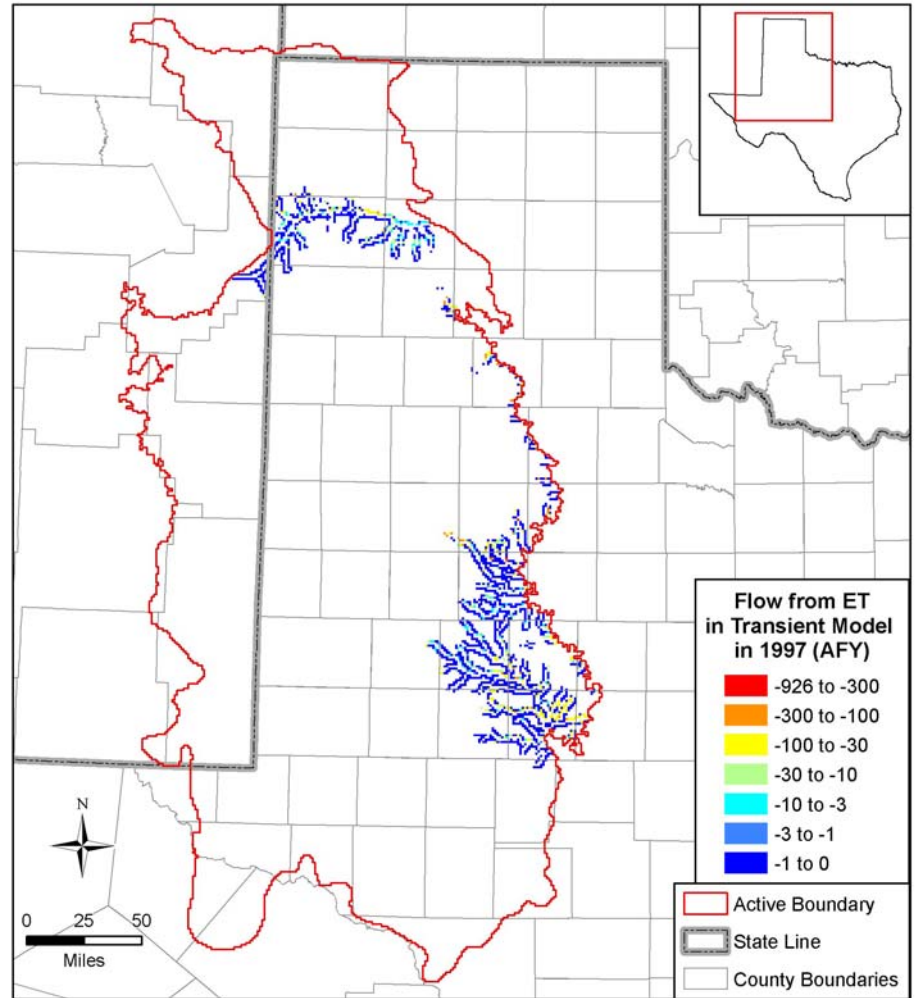
- **Many springs exhibit flow, however, some larger flows cannot be matched**
- **Lowering spring elevation or increasing conductance does not alleviate problem for larger springs**
- **Water is not being thieved by ET or streams**
- **Model inflows from recharge and from overlying younger units are consistent with the conceptual model**
- **Springs may be fed by high K seams/channels that are sub model-scale and cannot be simulated**

ET Flow

1990



1997

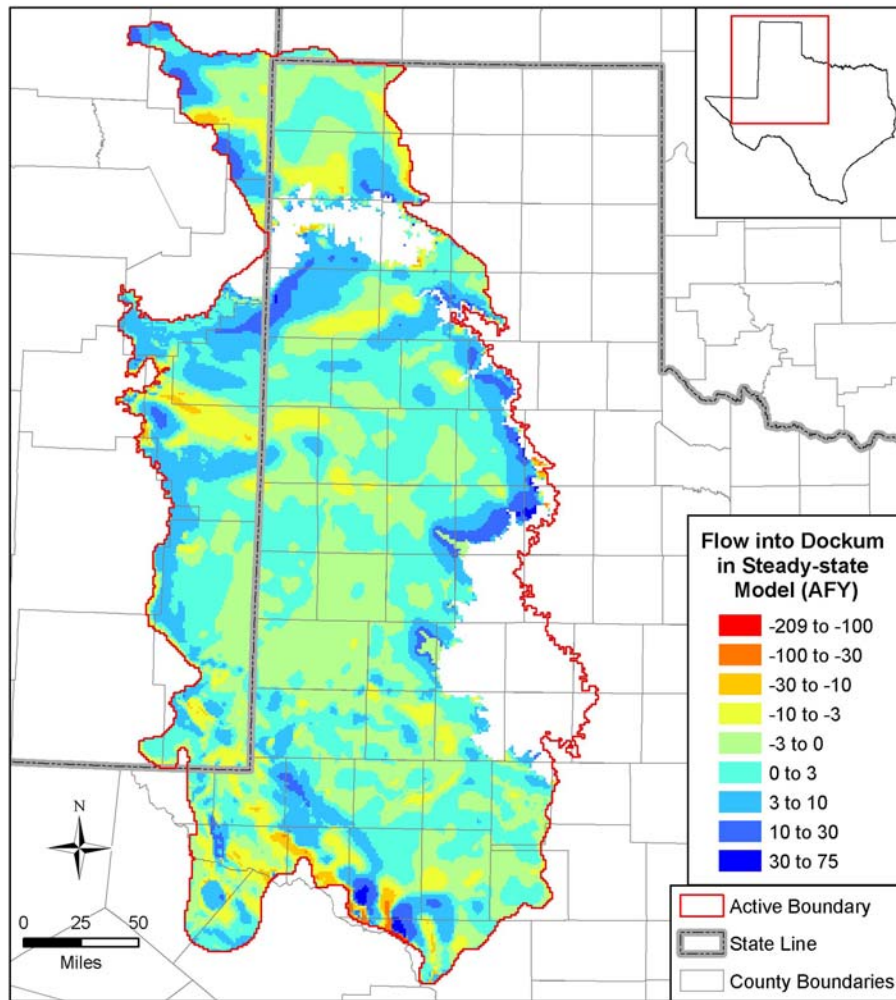


ET Issues

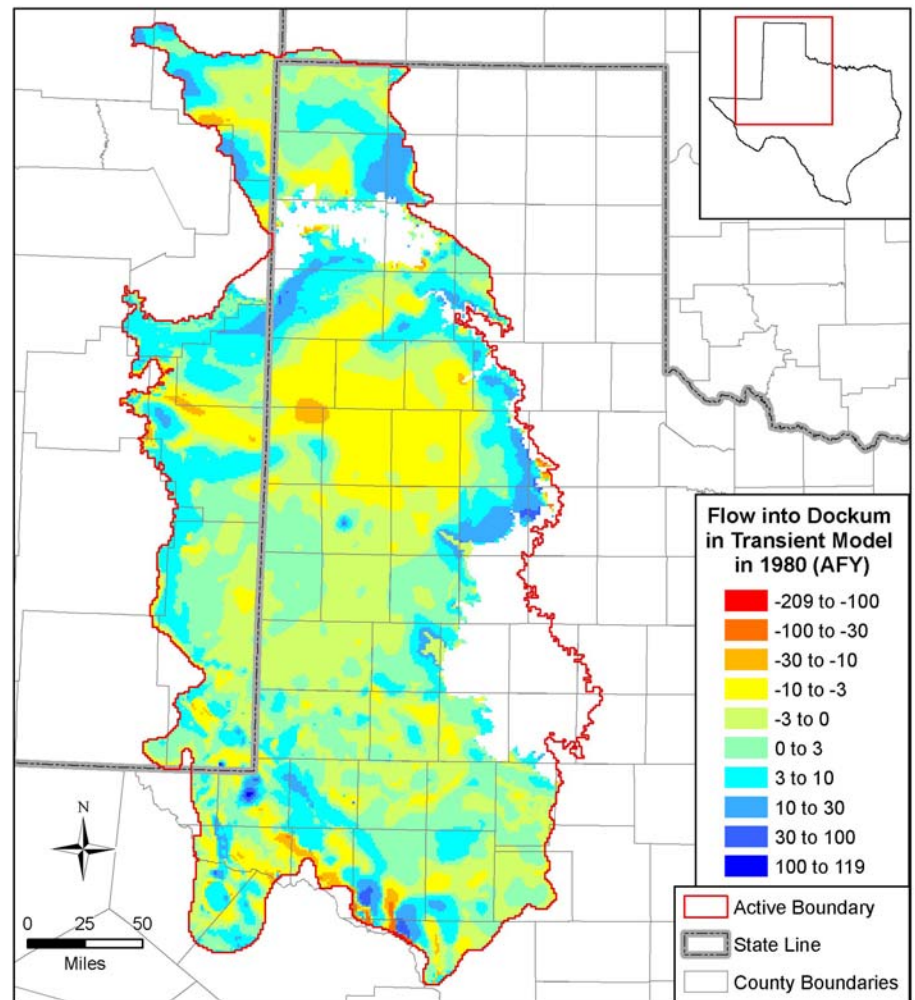
- **Calculated ET_{max} ranged from 32 to 52 in/yr**
- **With very large ET drain conductances, a maximum of 18 in/yr was simulated and generally much less**
- **ET flow is insensitive to drain conductance**
- **The Dockum formation is the limiter to ET flows**
- **The solver cannot handle relatively large rates being drawn from ET cells**
 - Cells dry out
 - Rewet option is invoked resulting in convergence issues

Flow into Top of Dockum

Predevelopment

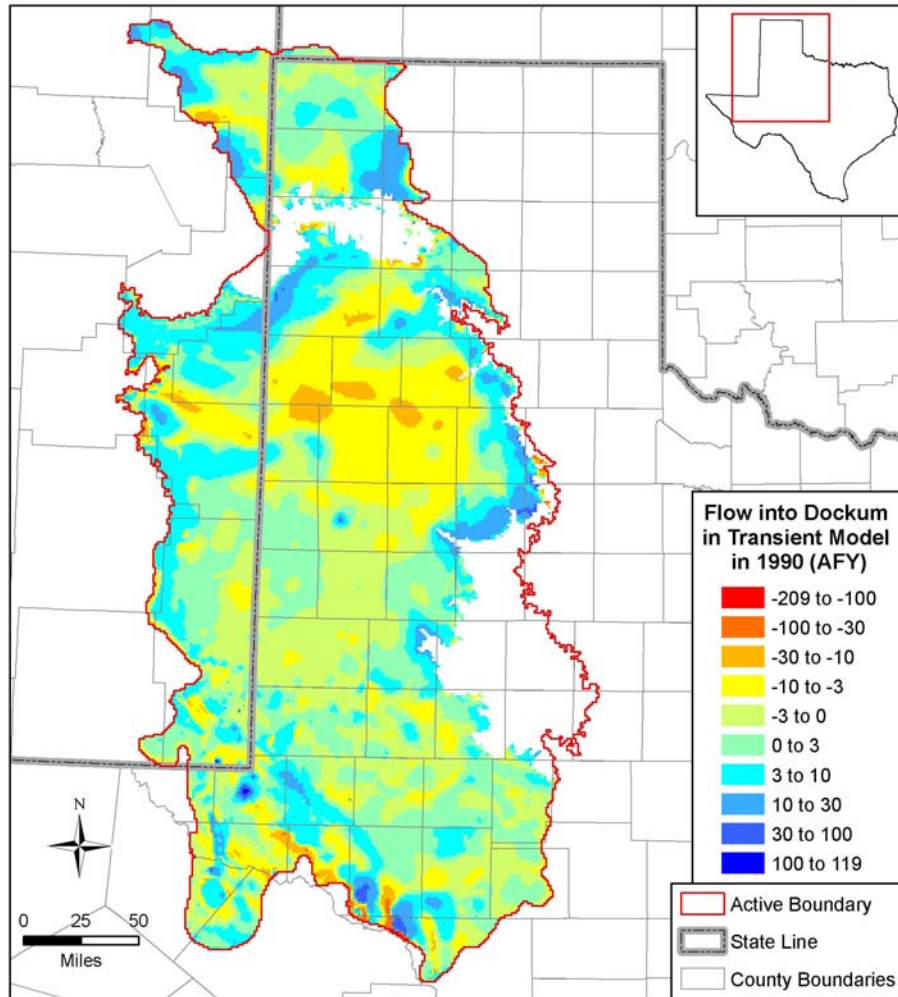


1980

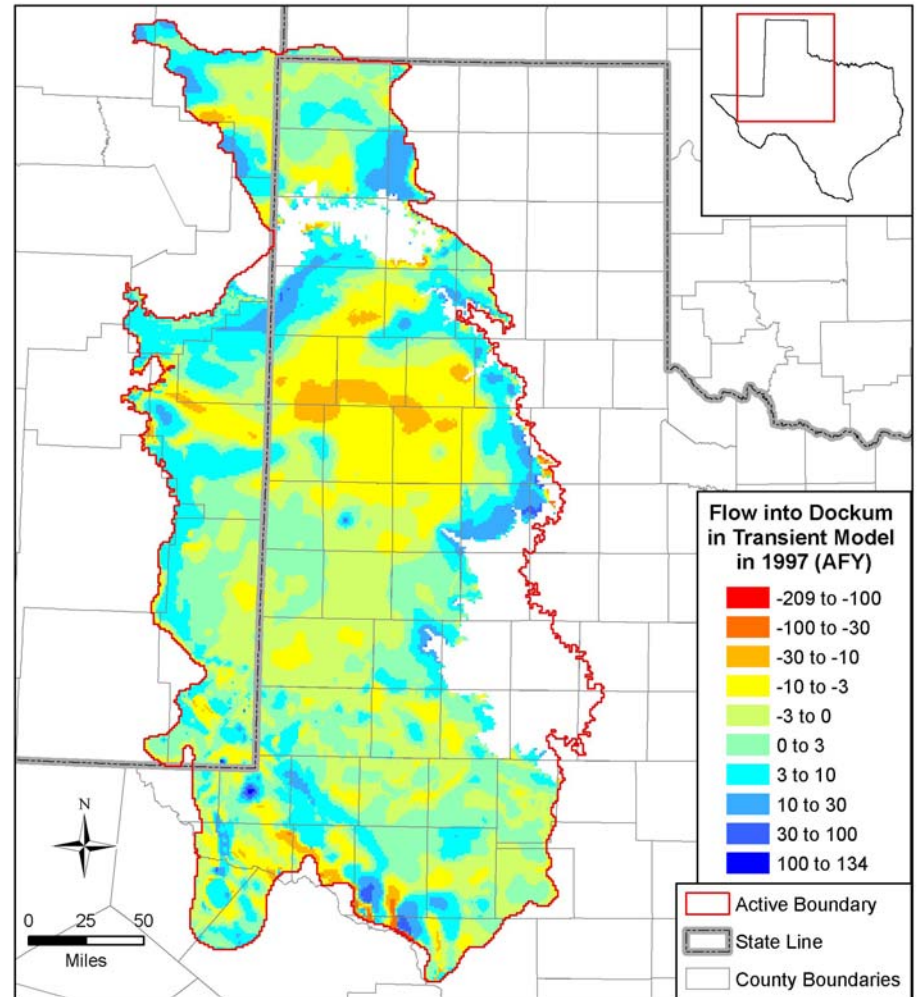


Flow into Top of Dockum

1990

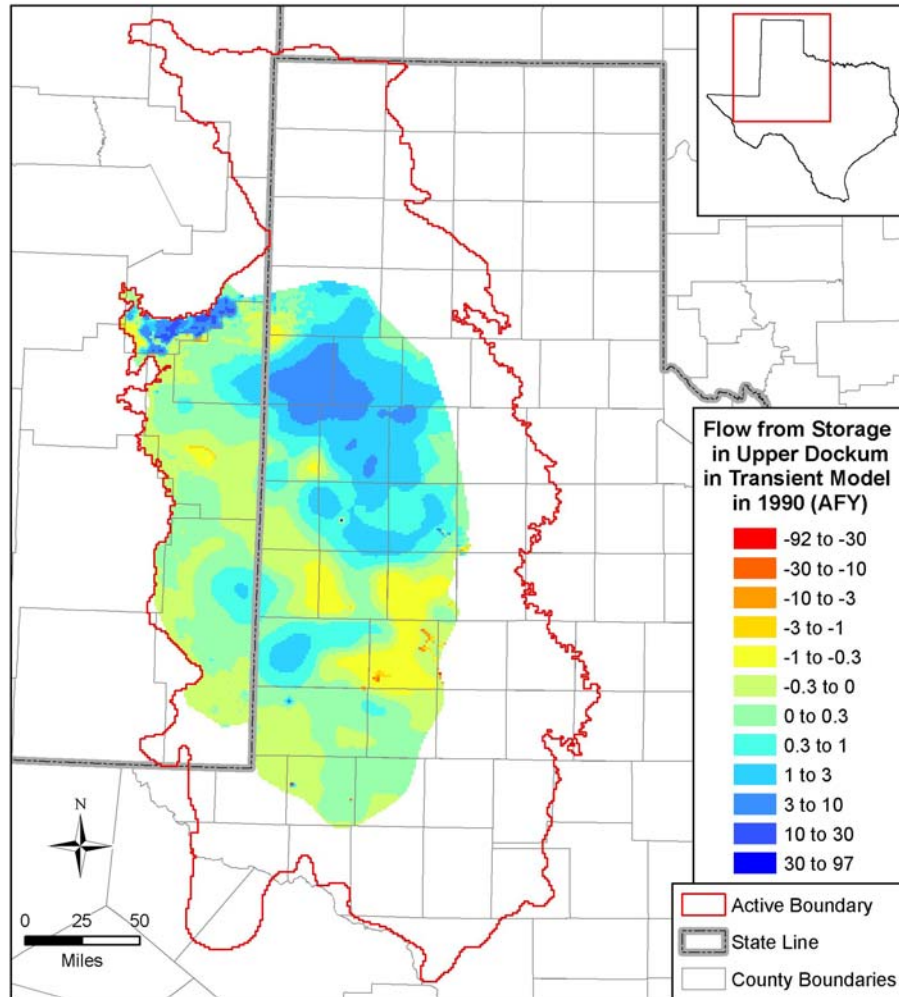


1997

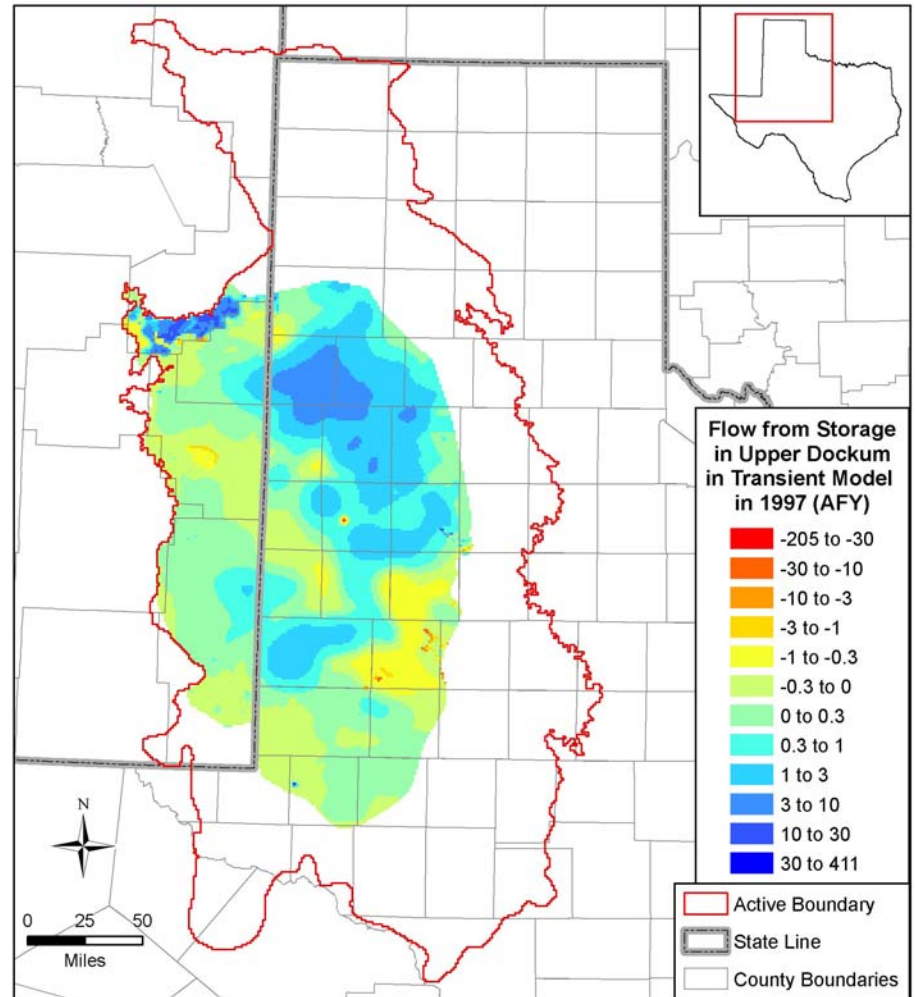


Flow from Storage in Upper Dockum

1990

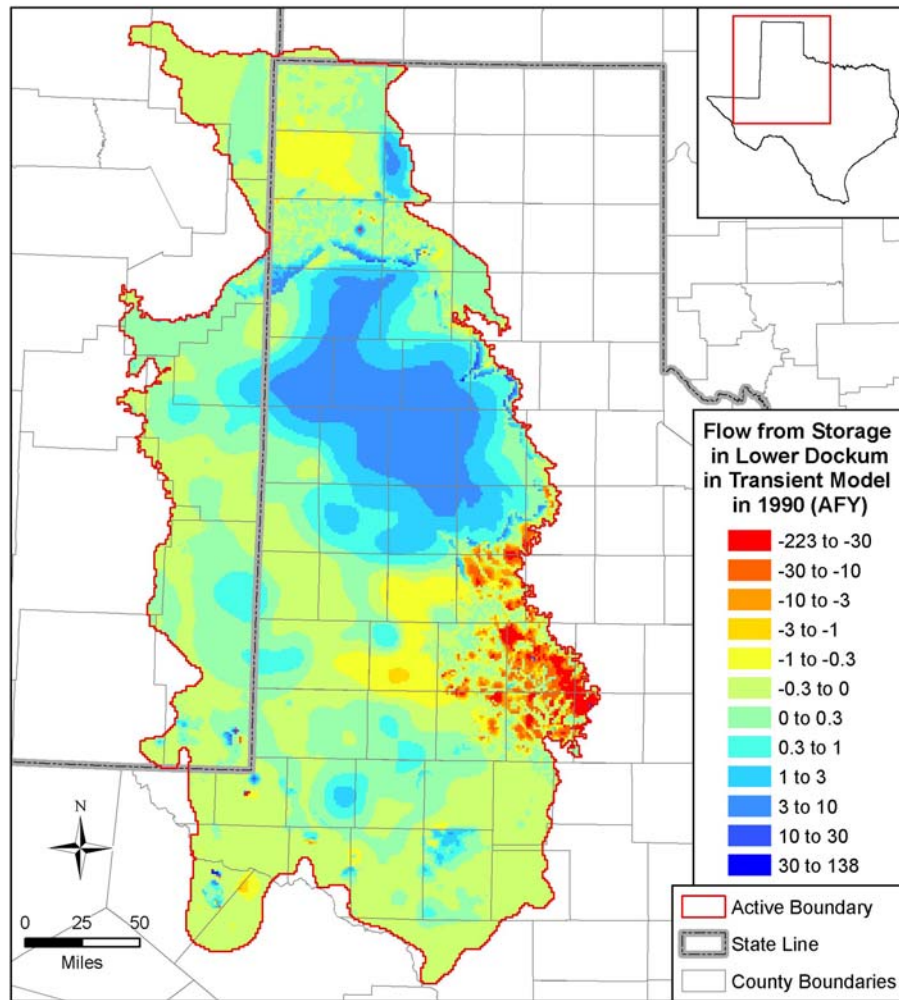


1997

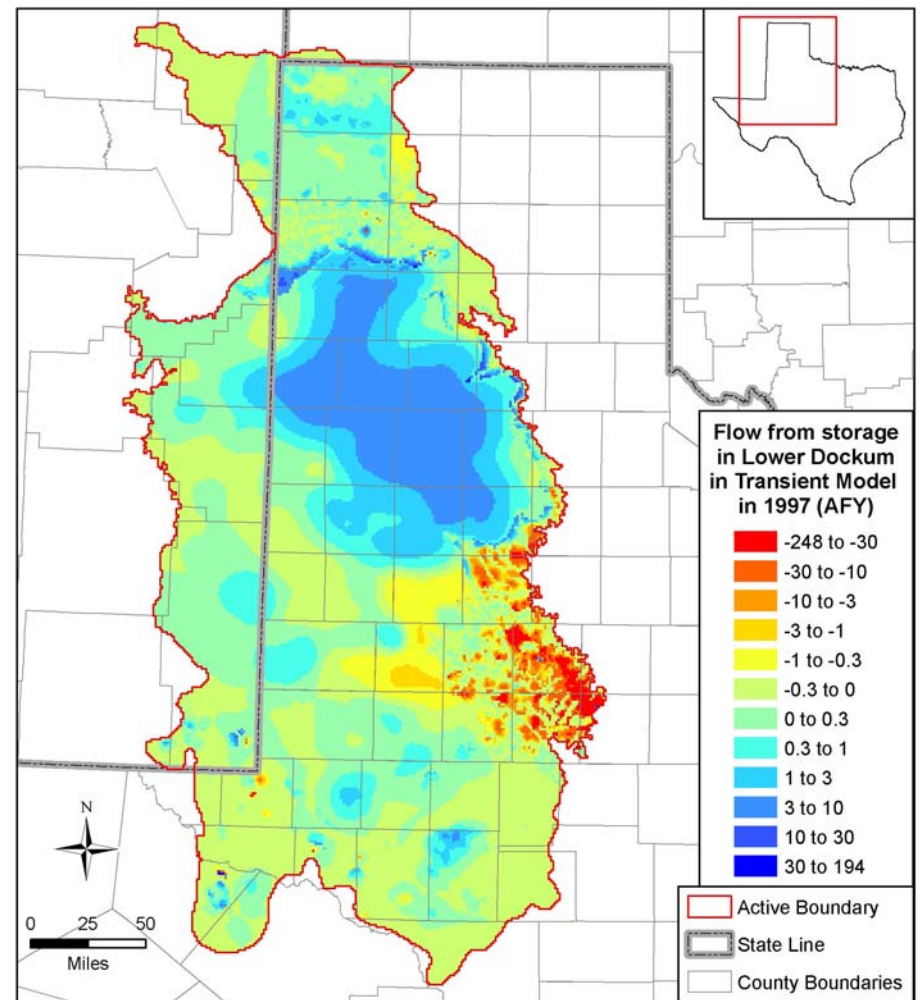


Flow from Storage in Lower Dockum

1990



1997



Dockum Mass Balance – pre-D & 1980

| pre-development | Inflow (AFY) | Outflow (AFY) | Net (AFY) | |
|---------------------|----------------|----------------|-----------|---------------|
| Recharge | 24,837 | 0 | 24,837 | ← 0.17 in/yr |
| X-Formational Upper | 38,617 | 18,596 | 20,020 | ← 0.015 in/yr |
| X-Formational Lower | 57,601 | 33,921 | 23,680 | ← 0.020 in/yr |
| Streams | 10,633 | 39,690 | -29,056 | |
| Springs | 0 | 1,446 | -1,446 | |
| Evapotranspiration | 0 | 38,044 | -38,044 | |
| Wells Upper | 0 | 0 | 0 | |
| Wells Lower | 0 | 0 | 0 | |
| Storage Upper | 0 | 0 | 0 | |
| Storage Lower | 0 | 0 | 0 | |
| Total | 131,688 | 131,697 | -9 | |
| Discrepancy | | | -0.01% | |

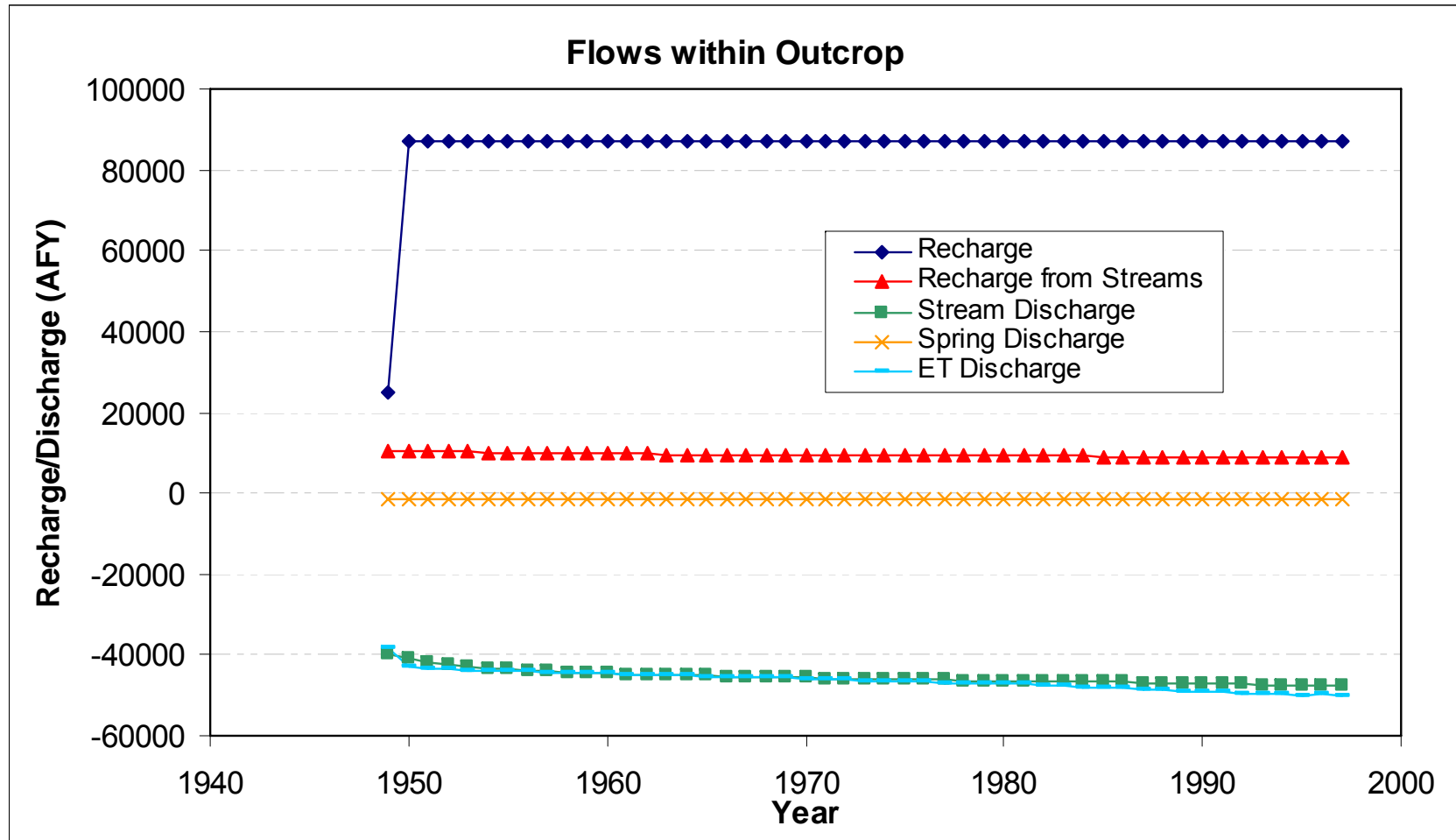
| 1980 | Inflow (AFY) | Outflow (AFY) | Net (AFY) | |
|---------------------|----------------|----------------|------------|----------------|
| Recharge | 87,167 | 0 | 87,167 | ← 0.58 in/yr |
| X-Formational Upper | 38,384 | 41,476 | -3,092 | ← -0.002 in/yr |
| X-Formational Lower | 74,737 | 31,573 | 43,164 | ← 0.036 in/yr |
| Streams | 9,235 | 46,368 | -37,133 | |
| Springs | 0 | 1,475 | -1,475 | |
| Evapotranspiration | 0 | 46,996 | -46,996 | |
| Wells Upper | 0 | 9,713 | -9,713 | |
| Wells Lower | 0 | 39,083 | -39,083 | |
| Storage Upper | 17,516 | 2,316 | 15,200 | |
| Storage Lower | 29,666 | 37,551 | -7,885 | |
| Total | 256,705 | 256,551 | 155 | |
| Discrepancy | | | 0.06% | |

Dockum Mass Balance – 1990 & 1997

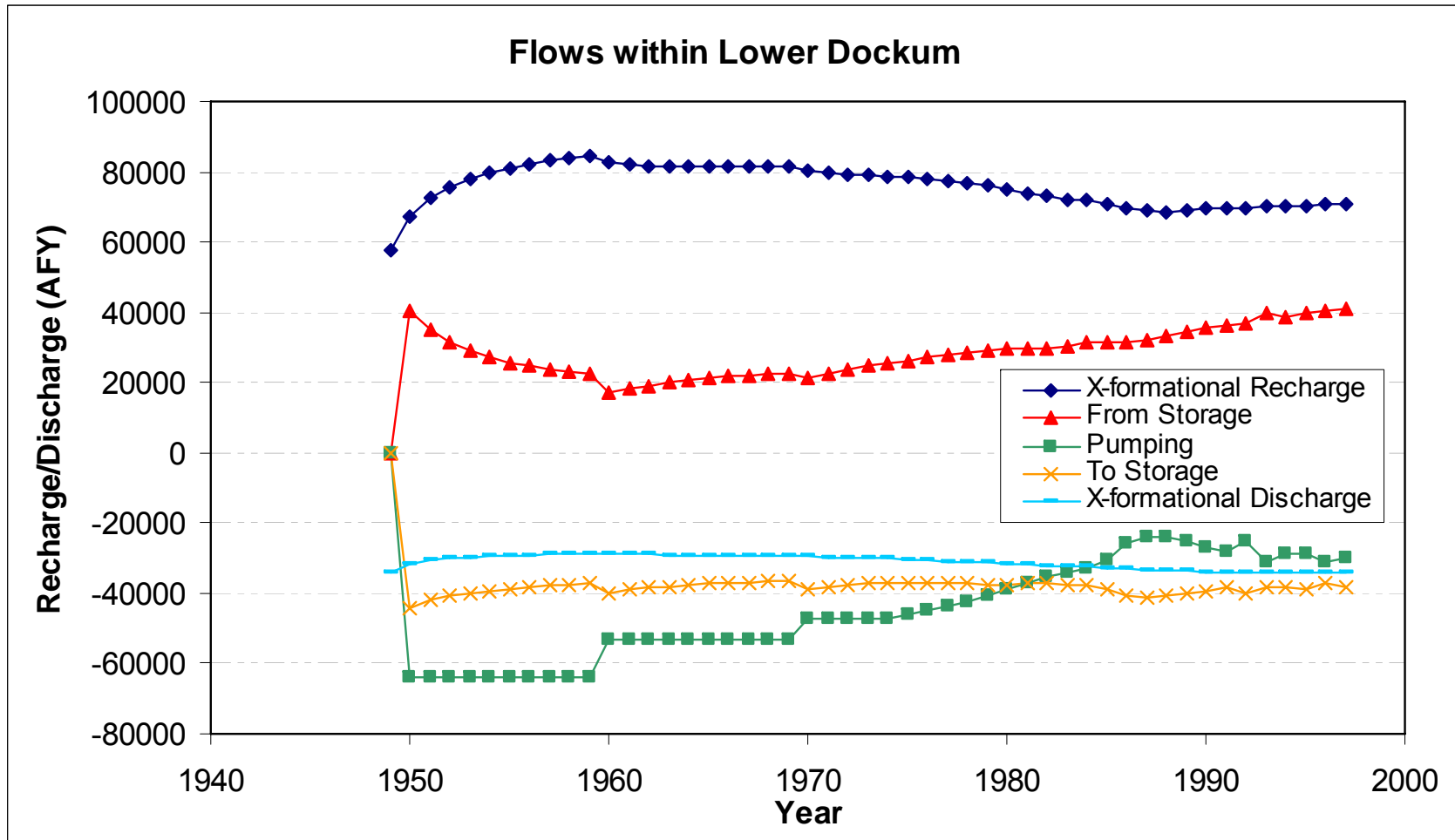
| 1990 | Inflow (AFY) | Outflow (AFY) | Net (AFY) | |
|---------------------|----------------|----------------|-----------|----------------|
| Recharge | 87,167 | 0 | 87,167 | ← 0.58 in/yr |
| X-Formational Upper | 38,077 | 49,254 | -11,178 | ← -0.008 in/yr |
| X-Formational Lower | 69,664 | 33,847 | 35,818 | ← 0.030 in/yr |
| Streams | 9,046 | 47,153 | -38,106 | |
| Springs | 0 | 1,478 | -1,478 | |
| Evapotranspiration | 0 | 49,220 | -49,220 | |
| Wells Upper | 0 | 8,729 | -8,729 | |
| Wells Lower | 0 | 26,878 | -26,878 | |
| Storage Upper | 17,918 | 1,634 | 16,284 | |
| Storage Lower | 35,835 | 39,481 | -3,645 | |
| Total | 257,707 | 257,673 | 34 | |
| Discrepancy | | | 0.01% | |

| 1997 | Inflow (AFY) | Outflow (AFY) | Net (AFY) | |
|---------------------|----------------|----------------|-----------|----------------|
| Recharge | 87,167 | 0 | 87,167 | ← 0.58 in/yr |
| X-Formational Upper | 38,312 | 52,737 | -14,426 | ← -0.011 in/yr |
| X-Formational Lower | 70,810 | 33,989 | 36,820 | ← 0.031 in/yr |
| Streams | 8,932 | 47,547 | -38,615 | |
| Springs | 0 | 1,478 | -1,478 | |
| Evapotranspiration | 0 | 50,087 | -50,087 | |
| Wells Upper | 0 | 9,197 | -9,197 | |
| Wells Lower | 0 | 29,955 | -29,955 | |
| Storage Upper | 18,735 | 1,624 | 17,111 | |
| Storage Lower | 40,898 | 38,217 | 2,681 | |
| Total | 264,854 | 264,833 | 21 | |
| Discrepancy | | | 0.01% | |

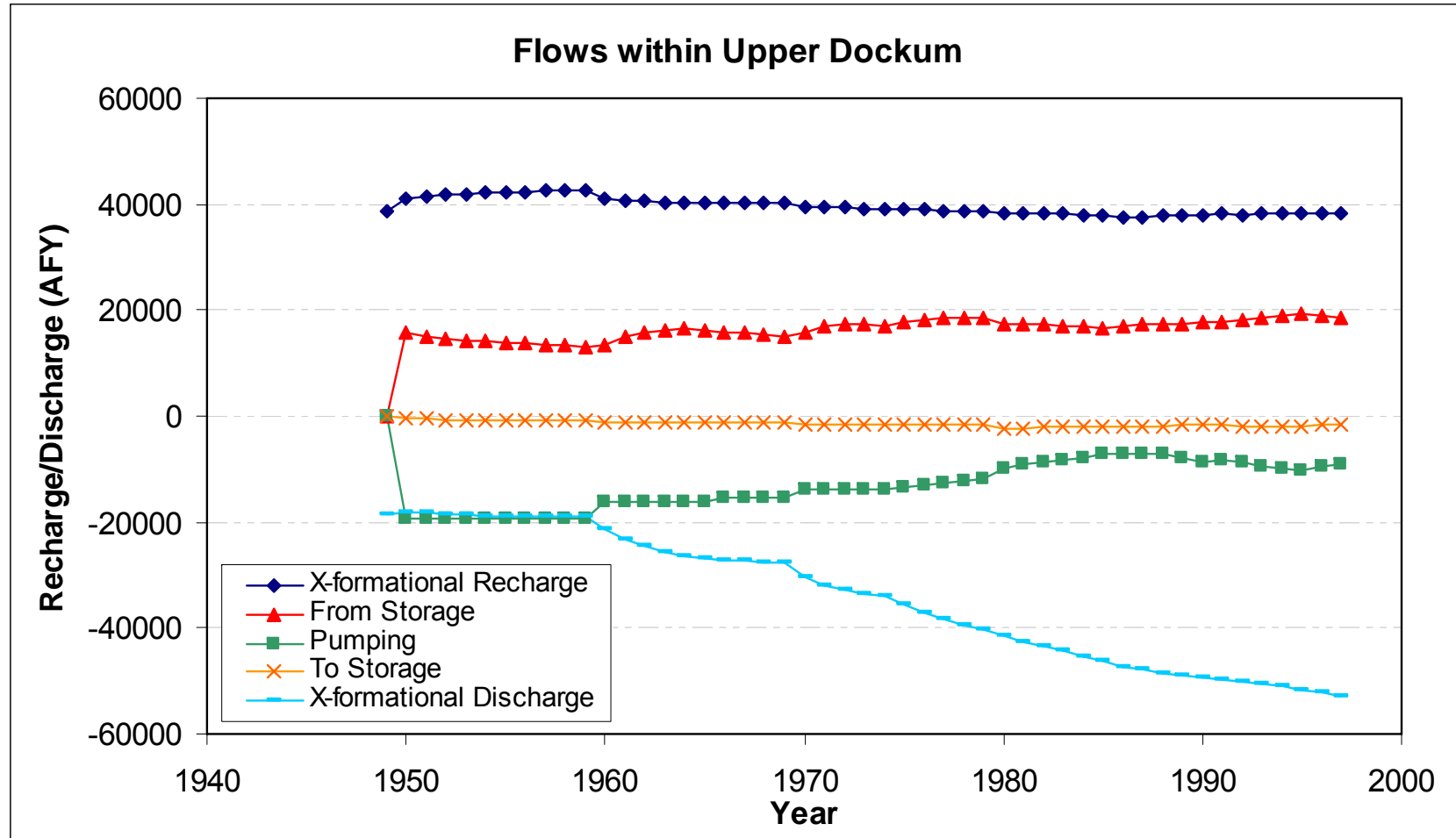
Transient Water Balance – Outcrop



Transient Water Balance – Subcrop



Transient Water Balance – Subcrop



Model Conclusions

- Recharge within the outcrop and cross-formational flow from younger formations are the largest sources of inflow to Dockum followed by stream losses
- Pumping, evapotranspiration, and stream gains are the significant discharge mechanisms
- Model is sensitive to K_H of Lower Dockum and GHB head
- Upper Dockum heads are sensitive to K_V of Upper Dockum
- Surface water discharge is sensitive to K_H of Lower Dockum, recharge and specific BC elevations

Model Limitations

- Dockum is a “minor” aquifer underlying “major” aquifers
- Regional model with groundwater availability prediction applicable at approximately county scale
- Temporal stress periods of one year preclude prediction of short-term head/flow variability
- The model does not provide a rigorous solution to surface water modeling
- Large portions of the Upper Dockum unconstrained by observed water-level data

GAM Schedule

- Project start – May 24, 2006
- Draft concept. Model mtg. – April 26, 2007
- Draft conceptual model report – May 16, 2007
- Steady-state model calibration mtg. – March 28, 2008
- Transient calibration & verification mtg. – May 20, 2008
- SAF3 Model calibration – June 4, 2008
- Draft Model Report to TWDB – June 30, 2008
- TWDB feedback on Draft Report – August 31, 2008
- Post draft final report review mtg. – September 2008
- Model Training Seminars – September 2008
- Final Model Report to TWDB – October 31, 2008

Dockum GAM
3rd Stakeholder Advisory Forum
June 4, 2008
Lubbock, TX

| Name | Affiliation |
|-----------------------|---------------------|
| Melanie Barnes | LWV |
| Ray Brady | RMBJ Geo Inc. |
| H.P. "Bo" Brown | Region D |
| Jason Coleman | South Plains UWCD |
| Steve and Nan Coneway | City of Hereford |
| Jim Conkwright | HPWD |
| Amy Crowell | PGCD |
| Harvey Everheart | Mesa UWCD |
| John Ewing | INTERA |
| Michelle Guelker | Lone Wolf GCD |
| Kevin Hopson | DBS&A |
| Ian Jones | TWDB |
| Mike McGregor | Llano Estacado UWCD |
| Bret Mills | Security State Bank |
| John Pickens | INTERA |
| Don M. Reynolds | HPWD |
| Julie Weathers | TTUHSC |
| Ben Weinheimer | TCFA |
| Chris Wingert | CRMWD |

Dockum Aquifer GAM
3rd Stakeholder Advisory Forum
Comments and Responses
June 4, 2008
Lubbock, Texas

Questions and Answers:

Q. Flow data control points – where available to define the Dockum structure contours?

A. We have the data control points in the figure used in the Conceptual Model Report and this level of detail will be provided in the Final Report.

Q. Moore County – are they combined, Ogallala and Dockum wells?

A. Yes, likely. We attempted to remove dual completion wells from model calibration.

Q. Have Santa Rosa wells drilled in Deaf Smith County in past 5 to 7 years been included in the model?

A. Yes, if the well property data has been submitted to the TWDB for posting on their website prior to our downloading of TWDB database for development of conceptual model report.

Note that the model calibration period is from 1980 to 1997, so water level data from last 5 to 7 years would not be included, but prior data would be included.

Details on the database and conceptual model were presented at SAF2 and a CM report was prepared and posted. All data will be included in the Final Report.

Q. What is the source of pumping data in NM?

A. Numbers for pumping by county were taken from either the New Mexico Office of the State Engineer (NM OSE) (for 1975, 1980, 1990, 1995, and 2000) or the USGS (for 1985). Irrigation and Livestock pumping was distributed by land use type from the National Land Cover Data (NLCD) map. Rural Domestic pumping was distributed by population density. Municipal, Industrial, and Manufacturing well locations were taken from the NM OSE. For Dockum/Ogallala and Dockum/Pecos wells, 25% of the pumping was applied to the Dockum.

Q. Question on the reliability of the 5,000 mg/l aquifer limit definition.

A. There is sufficient data to reasonably define the 5,000 mg/L TDS boundary. The location is uncertain and based on limited data in some areas, however. It is a gradual decline in concentration from 5,000 mg/L and, because the high TDS area is being included in the active model domain, it is really only a qualitative threshold for reference rather than anything included in the MODFLOW model.

Q. For Deaf Smith County model area, should pumping be in Lower Dockum rather than Upper Dockum?

A. Yes. After reviewing the methodology for pumping layer assignments, it became clear that all the pumping applied to the Upper Dockum throughout the entirety of the model domain should be applied instead to the Lower Dockum. Because of the relatively small amount of this pumping, the change had minimal impact on the model results.

Q. What does “Water is not thieved by ET or streams” mean?

A. This was just one of the tests to identify the mechanisms limiting spring flow in the model. It turns out that the Dockum aquifer properties (not the spring properties or nearby stream and ET properties) limit the spring discharge rate in the model. We are aware that ET occurs in reality and have included it in the model for this reason.

Q. Deaf Smith County pumping issue?

A. See prior answer regarding reassignment of pumping to Lower Dockum.

Q. Should there be a separation of Upper and Lower Dockum?

A. While there may be uncertainty in the interface between the Upper and Lower Dockum, the two units have significant differences in hydraulic properties and stresses. By separating the two units into model layers, a more accurate representation of the conceptual model is possible.

Q. If asked to look at decline of 1 ft, 2 ft, etc. per year in Deaf Smith County, how usable is model?

A. It is applicable to apply the model at the county scale. However, limited water level data in Deaf Smith county during the calibration period means that the model is not well constrained by data in that area.

Q. Good to include high TDS region in model because of future desalinization potential. Should there be caution in using data points 80 miles apart?

A. This area of the model is certainly poorly constrained data. However, the model imposes physical constraints based on the reasonable estimates of hydraulic properties and structure and the equations governing groundwater flow so it may be a useful estimation tool. It should be noted, though, that MODFLOW does not account for density dependant flow as a function of salinity.

Q. We would like to see where data is limited (e.g., dashed lines).

A. In the Final Report attention will be given to clearly delineate areas where data is insufficient for contouring.

Q. Can we use more color consistency between figures showing similar information?

A. In the Final Report we will attempt to provide consistent color scales between all comparable figures so that they may be more efficiently compared visually.